# STEVENS FARM LUZERNE COUNTY, PA LOCATION MAP:



# CONSTRUCTION NOTES

- CLEAR AND GRUB THE ENTIRE AREA WITHIN THE WORK LIMITS
- ALL FILL MATERIAL MUST NOT CONTAIN FROZEN MATERIAL. SOD, ROOTS, OR OTHER PERISHABLE MATERIAL, OR ROCK LARGER THAN 8" IN DIAMETER.
- 6" OF TOPOSOIL WILL BE INCORPORATED INTO THE EARTHFILL TO MEET THE NEAT LINES SHOWN ON THE TYPICAL SECTION.
- ALL AREAS DISTURBED DURING CONSTRUCTION WILL BE SEEDED ACCORDING TO NRCS CRITICAL AREA PLANTING SPECIFICATION.

WATER WELL

LDG TAKES SAFETY VERY SERIUOUSLY, HOWEVER, THE SAFETY COMMITMENT AND THE JOB SITE PRACTICES OF THE CONTRACTOR ARE BEYOND THE CONTROL OF LDG. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP ENSURE WORKER SAFETY. MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF CONSTRUCTING THE DESIGNED PRACTICES. EMERGENCY PROCEDURES ARE ALSO RECOMMENDED. IT IS THE CONTRACTORS RESPONSIBILITY TO ENSURE A SAFE WORK ENVIRONMENT FOR THEIR EMPLOYEES.

# **GENERAL NOTES**

1. ALL FEDERAL, STATE, AND LOCAL LAWS, RULES, AND REGULATIONS GOVERNING THE CONSTRUCTION OF THIS FACILITY SHALL BE STRICTLY FOLLOWED. THE OWNER OR OPERATOR IS RESPONSIBLE FOR OBTAINING ALL CONSTRUCTION PERMITS.

IT IS THE RESPONSIBILITY OF THE EXCAVATING CONTRACTOR TO COMPLY WITH PA ACT 187 (1996) AND ALL ITS REVISIONS BEFORE PERFORMING ANY EXCAVATION. THE PA ONE-CALL PHONE NUMBER IS 1(800)-242-1776. THE SERIAL NUMBER FOR DESIGN IS 20250171484 DATED 1/17/2025 CONTRACTOR TO PROVIDE ONE-CALL FOR CONSTRUCTION.

- 2. A MEETING BETWEEN THE LANDOWNER, CONTRACTOR, LUZERNE CONSERVATION DISTRICT REPRESENTATIVE, AND ENGINEER SHALL BE REQUIRED PRIOR TO ANY EXCAVATION OR CONSTRUCTION WORK.
- 3. A COPY OF THE SPECIFICATIONS AND DRAWINGS SHALL BE ONSITE DURING ALL PHASES OF CONSTRUCTION.
- 4. OSHA REGULATIONS SHALL BE FOLLOWED AT ALL TIMES.

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CONTROL JOINT PLAN

SITE PLAN

GRADING PLAN

BUILDING DRAINAGE PLAN

EROSION AND SEDIMENT CONTROL DETAILS

EROSION AND SEDIMENT CONTROL PLAN

SITE DETAILS

SITE DETAILS

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- THE CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTING ALL MEASURES NECESSARY TO PROTECT WORK IN PROGRESS FROM ENVIRONMENTAL CONDITIONS SUCH AS TEMPERATURE EXTREMES, SURFACE, AND GROUND WATER
- 6. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ACTUAL FIELD MEASUREMENTS SHOWN ON THE PLANS.
- 7. IN THE EVENT ROCK, UNSTABLE SOILS, OR SEEPS ARE ENCOUNTERED DURING EXCAVATION, WORK SHALL BE STOPPED AND THE ENGINEER SHALL DETERMINE HOW TO PROCEED.
- THE CONTRACTOR IS RESPONSIBLE FOR THE SECURITY OF THE JOB SITE UNTIL THE WORK HAS BEEN CERTIFIED BY THE ENGINEER.
- 10. CERTIFICATION OF CONFORMANCE SHALL CERTIFY THAT ALL WORK WAS PERFORMED TO DESIGN SPECIFICATIONS.
- 11. THE OWNER IS RESPONSIBLE FOR ENSURING THAT ALL LIVESTOCK ARE REMOVED FROM THE WORK SITE AND THAT LIVESTOCK WILL REMAIN EXCLUDED FROM THE WORK SITE UNTIL THE PROJECT HAS BEEN THROUGH A FINAL CERTIFICATION AND APPROVED FOR USE. TEMPORARY LIVESTOCK CONFINEMENT/EXCLUSION FENCE MAY BE NEEDED TO ENSURE LIVESTOCK ARE NOT ABLE TO ENTER THE WORK SITE.
- 12. EMERGENCY RESPONSE STRATEGIES FOR MANURE SPILLS ARE NECESSARY. CONTACT INFORMATION FOR EMERGENCIES SHOULD BE INCLUDED IN THE EMERGENCY RESPONSE SECTION OF THE NUTRIENT MANAGEMENT PLAN AND IN OPERATION AND MAINTENANCE PLANS FOR BEST MANAGEMENT PRACTICES (BMP'S).

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**PROJECT** 

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**STEVENS** Date: 2025-01-31 Project No.

13655-002

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		QUALITY ASSURANCE S	TATEMENT	ENGINEER STATEMENT				
To the best of my knowledge, I certify that the practices have been installed as per the attached drawings and specifications, based on the information provided to me and/or observations I have made.					In my professional opinion, I certify that the practices have been installed as per the attached drawings and specifications, based on the information provided to me and/or observations I have made.			
Practice Code	CIN	Description	Planned Amount	Inspector (Initials)	As-Built Amount (by Inspector)	Certification (Engineer/JAA Signature)	Date Certified	
313		WASTE STORAGE FACILITY	1 QTY.					
367		ROOFS AND COVERS	1 QTY.					
468		PROTECTED OUTLET	5.5 S.Y					
516		LIVESTOCK PIPELINE	140 L.F					
533		PUMPING PLANT	1 QTY.					
558		GUTTERS/DOWNSPOUTS	425 L.F					
560		ACCESS ROAD	1,900 S.Y					
561		HUA PROTECTION	8,000 S.F					
614		WATERING FACILITY	2 WATERERS					
620		UNDERGROUND OUTLET	900 L.F					

1 QTY.

AS-BUILT / DESIGN INFORMATION

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# **OWNER RESPONSIBILITIES**

- THE OWNER IS RESPONSIBLE FOR ENSURING THAT ALL LIVESTOCK ARE REMOVED FROM THE WORK SITE AND THAT LIVESTOCK WILL REMAIN EXCLUDED FROM THE WORK SITE UNTIL THE PROJECT HAS RECEIVED FINAL CERTIFICATION AND IS APPROVED FOR USE.
- 2. THE OWNER IS TO PROVIDE REASONABLE ACCESS TO THE WORK SITE.
- 3. THE OWNER OR OPERATOR IS RESPONSIBLE FOR OBTAINING ALL CONSTRUCTION PERMITS.

# **EXCAVATION NOTES**

- NO EXCAVATION SHALL BEGIN UNTIL THE EXCAVATOR HAS COMPLIED WITH ALL PA ONE-CALL REQUIREMENTS AND ANY UTILITY COMPANY RESPONSES.
- ALL EROSION AND SEDIMENT PRACTICES SHALL BE INSTALLED PRIOR TO BEGINNING EXCAVATION.
- OSHA STANDARDS SHALL BE FOLLOWED FOR ALL EXCAVATION.
- TOPSOIL SHALL BE STRIPPED AND STOCKPILED TO BE RE-DISTRIBUTED WHEN THE PROJECT IS COMPLETE.
- ALL MANURE-LADEN SOIL SHALL BE REMOVED AND SPREAD ACCORDING TO THE LANDOWNER'S NUTRIENT MANAGEMENT PLAN.
- THE SITE SHALL BE EXCAVATED UNTIL GOOD, STABLE SOIL IS ENCOUNTERED,
- IF SEEPS ARE ENCOUNTERED DURING EXCAVATION, PROVIDE CLEAN 2B-STONE BACKFILL UP TO THE SEEP ELEVATION.
- IF ROCK REFUSAL IS MET BEFORE DESIGN SUBGRADE, CHANGES IN DESIGN ELEVATIONS WILL REQUIRE DESIGNER AND LANDOWNER APPROVAL.
- 9. EXCESS MATERIAL SHALL BE DISPOSED OF AS DIRECTED BY THE LANDOWNER AND THE INSPECTOR.
- 10. A UNIFORM LAYER OF 2B-STONE (AASHTO #57), 3" THICK SHALL BE PLACED ABOVE SUBGRADE TO BED ALL CONCRETE. STONE DEPTH TO BE MEASURE AFTER COMPACTION. STONE SHALL NOT BE PLACED UNTIL EARTHEN SUBGRADE ELEVATION AND COMPACTION IS APPROVED BY THE INSPECTOR.
- 11. ALLOW 1' OVERLAP BETWEEN ADJACENT PANELS OF GEOTEXTILE WHERE APPLICABLE
- 12. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE CONSTRUCTION SITE UNTIL THE WORK HAS BEEN COMPLETED AND CERTIFIED BY THE DESIGN ENGINEER. THIS INCLUDES DEWATERING THE SITE AS NECESSARY, AS WELL AS PREVENTING UPSLOPE RUNOFF FROM ENTERING THE WORK AREA. IT IS STRONGLY RECOMMENDED THAT ALL PLANNED DIVERSIONS OR SWALES BE INSTALLED FIRST AND ALL PERIMETER DRAIN OUTLETS BE INSTALLED BEFORE STONE OR CONCRETE IS PLACED, IF POSSIBLE.
- 13. FINAL GRADING SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM ALL STRUCTURES. SWALES SHALL BE SHAPED AS NECESSARY ALONG THE HEAVY USE AREA AND MANURE STORAGE TO DIRECT STORMWATER AWAY FROM THE STRUCTURES.

# EARTHFILL NOTES

- 1. EARTHEN BACKFILL SHALL BE PLACED IN A MANNER THAT PREVENTS DAMAGE TO THE STRUCTURES AND ALLOWS THE STRUCTURES TO ASSUME THE LOADS FROM THE EARTH BACKFILL GRADUALLY AND UNIFORMLY. THE HEIGHT OF THE EARTH BACKFILL ADJACENT TO THE STRUCTURE SHALL BE INCREASED AT THE SAME RATE ON ALL SIDES OF THE STRUCTURE.
- 2. BACKFILL SHALL BE PLACED IN EVEN, HORIZONTAL LAYERS. IF NECESSARY, OVER-EXCAVATE TO AN APPROXIMATELY LEVEL SURFACE AND BUILD SUBGRADE IN EVENLY COMPACTED, HORIZONTAL LIFTS OF SPECIFIED THICKNESS.
- BACKFILL SHALL BE PLACED AT OPTIMUM MOISTURE CONTENT. BACKFILLED MATERIAL SHALL HAVE ENOUGH MOISTURE SO THAT WHEN FORMED INTO A BALL, IT WILL NOT BREAK IF STRUCK SHARPLY WITH A PENCIL BACKFILLING NEWLY POURED WALLS MAY NOT BEGIN UNTIL 14-DAYS AFTER THE FINAL CONCRETE PLACEMENT. COMPACT USING THE FOLLOWING EQUIPMENT AND LIFT THICKNESS: FOOTINGS AND STRUCTURE FLOOR:
  - (3) PASSES OF SHEEPSFOOT OR VIBRATORY ROLLER IN 6-INCH LIFTS WITHIN 3 FEET OF WALLS: A ROLLER CAN BE USED BEYOND 3 FEET OF THE WALL WITHOUT THE VIBRATORY FUNCTION ON. DO NOT USE ANY VIBRATORY EQUIPMENT WITHIN A DISTANCE EQUAL TO THE WALL HEIGHT:
  - (3) PASSES BY HAND COMPACTOR OR SMALL, MANUALLY DIRECTED PLATE VIBRATOR IN 6-INCH LIFTS
  - BEYOND 3 FEET OF WALLS:
    - (3) PASSES BY TRACK EQUIPMENT (>4,000 LBS) IN 6-INCH LIFTS
    - (3) PASSES BY RUBBER TIRED EQUIPMENT IN 6-INCH LIFTS
    - (3) PASSES OF VIBRATORY ROLLER IN 6-INCH LIFTS
- 4. AVOID BACKFILL CONTAINING ROCKS OR CLODS GREATER THAN 3" DIAMETER, DEBRIS, ROOTS, FROZEN SOIL, OR OTHER UNSUITABLE MATERIAL AS DETERMINED BY THE NRCS INSPECTOR.
- IF SEEPS ARE ENCOUNTERED DURING EXCAVATION, PROVIDE CLEAN, ASHTO #57 STONE 1' ABOVE THAT ELEVATION AND EXTEND THE STONE A MINIMUM OF 4' LEFT/RIGHT OF THE SEEP LOCATION. IN ADDITION TO THE CONTROL MEASURES, IMPLEMENTATION OF CORRUGATED PIPE MAY ALSO BE NECESSARY.

# PIPE NOTES

- ALL PIPES SHALL MEET MINIMUM MATERIAL SPECIFICATIONS:
- 1.1. SCH 40 PVC SHALL MEET ASTM-D1785 1.2.SDR-35 SHALL MEET ASTM-D3034
- 1.3. CORRUGATED POLYETHYLENE TUBING SHALL MEET ASTM-F405
- 2. ALL FITTINGS SHALL BE PRESSURE-RATED, WATERTIGHT AND MEET MINIMUM MATERIAL SPECIFICATIONS OF PIPE.
- PIPES SHALL BE INSTALLED TO SPECIFIED DEPTH AND TO MINIMUM DESIGN GRADE.
- TRENCHES FOR PIPELINES SHALL BE FREE OF ROCKS AND SHARP-EDGED MATERIALS. A SUPPLY OF AASHTO #57 BEDDING, OR OTHER SUITABLE GRANULAR MATERIAL, SHALL BE AVAILABLE TO BED PIPELINES IN UNSTABLE SOILS OR AS DIRECTED BY NRCS INSPECTORS.
- 5. PIPES SHALL BE BACKFILLED AS SHOWN ON DESIGN DETAILS. ANY PIPE TO BE PLACED IN A TRAFFIC AREA IS TO BE BEDDED AS PER DESIGN DETAILS AND BACKFILLED TO THE SURFACE WITH 2A MODIFIED OR 2RC AGGREGATE. ANY PIPE NOT SPECIFICALLY DETAILED MAY BE BACKFILLED WITH MOIST EARTH, FREE OF LARGE CLODS OR ROCKS, AND HAND COMPACTED IN 6-INCH LIFTS. DO NOT DRIVE MACHINERY OVER RECENTLY BACKFILLED PIPES. MOUND BACKFILL 10% OF TRENCH DEPTH TO ALLOW FOR SETTLEMENT.

# PUMPING PLANT NOTES

PROVIDE 8'X8' INSULATED ROOM FOR PUMPING PLANT. COORDINATE LOCATION WITH LANDOWNER.

# **GEOTEXTILE NOTES**

1. SEE TABLE 1 AND 2, BELOW:

Property	Test	Method	Class I	Clas	s II & III	Class IV
Tensile strength (pounds) 1/	ASTN grab	MD4632 test	200 minimum in principal direction		ninimum in any pal direction	180 minimum in any principal direction
Elongation at fail (percent) <u>1/</u>	ure ASTN grab	MD4632 test	<50	<50		<50
Puncture (pounds) 1/	ASTN	MD4833	90 minimum	60 mi	nimum	60 minimum
Ultraviolet light (% residual tensi strength)		MD4355 ir exposure	70 minimum	70 mi	nimum	70 minimum
Apparent opening size (AOS)	g ASTN	MD4751	As specified, but no smaller than 0.212 mm(#70)	no sn	ecified, but naller than mm(#70) <u>2</u> /	As specified, but no smaller than 0.212 mm(#70) 2/
Percent open are	a CWO	-02215-86	4.0 minimum	4.0 m	inimum	1.0 minimum
(percent)						
(percent)  Permitivity sec -1  // Minimur  // U.S. sta  Note: CWO is a	n average roll v ndard sleve size USACE referen	e ce	0.10 minimum est principal direc	ion).	ninimum	0.10 minimum
(percent)  Permitivity sec -1  // Minimur  // U.S. sta  Note: CWO is a	n average roll v ndard sleve size USACE referen	alue (weake e ce nts for ne		ion).	ninimum Class III	0.10 minimum
(percent)  Permitivity sec -1  1/ Minimur  2/ U.S. sta  Note: CWO is a  Table 2  Property  Tensile strength	n average roll v indard sieve size USACE referen Requireme Test Method	alue (weake e ce nts for ne	est principal direc	tion).		
(percent)  Permittivity sec -1  1/ Minimur  2/ U.S. sta  Note: CWO is a  Table 2  Property  Tensile strength (b) 1/  Elongation at	m average roll vindard sieve size USACE referen  Requireme Test Method  ASTMD 4632	alue (weake e ce nts for ne	est principal directions on woven geo	textiles Class II	Class III	Class IV <u>3</u> /
(percent)  Permitivity sec -1  1/ Minimum  2/ U.S. sta  Note: CWO is a  Table 2  Property  Tensile strength  (ib) 1/  Elongation at failure (%) 1/  Puncture	m average roll v madard sleve size USACE referen Requireme Test Method ASTMD 4632 grab test	alue (weake e ce nts for ne	onwoven geo Class I	textiles Class II 120 minimum	Class III 90 minimum	Class IV <u>3/</u> 115 minimum
(percent)  Permittivity sec -1  1/ Minimur  2/ U.S. sta Note: CWO is a  Table 2  Property  Tensile strength (b) 1/  Elongation at failure (%) 1/  Puncture (pounds)  Ultraviolet light (% residual	n average roll v ndard sieve size USACE referen Requireme Test Method ASTMD 4632 grab test ASTMD 4632	alue (weake e ce nts for no	onwoven geo Class I 180 minimum ≥50	textiles Class II  120 minimum ≥50	Class III 90 minimum ≥50	Class IV <u>3</u> / 115 minimum ≥50
(percent) Permitivity sec - <u>1</u> 1/ Minimur	m average roll v ndard sieve sizu USACE referen Requireme Test Method ASTMD 4632 grab test ASTMD 4632 ASTMD 4833	alue (weake e ce nts for no	onwoven geo Class I  180 minimum  ≥50  80 minimum	textiles Class II  120 minimum  ≥50  60 minimum	Class III 90 minimum ≥50 40 minimum	Class IV <u>3</u> / 115 minimum ≥50 40 minimum

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# CONCRETE CONSTRUCTION NOTES

## REINFORCEMENT

- REINFORCING STEEL IS TO BE GRADE 60. WHERE 6"X6" W2.9XW2.9 (6 GAGE) IS SPECIFIED; THE FABRIC SHALL BE MATS, NOT ROLLS, SUPPORTED ON STEEL CHAIRS. NO CINDER OR CONCRETE BRICKS ARE PERMITTED. SUPPORT SHALL BE OFTEN ENOUGH SO REINFORCEMENT STAYS AT THE REQUIRED LOCATION WITHIN THE SLAB OR FOOTING. A 5' (MAX) CHAIR SPACING IS REQUIRED.
- 2. FORM OIL SHALL NOT BE SPRAYED ON ANY REBAR, WATERSTOPS, OR CONCRETE.

## CONCRETE

- 4,000 PSI 28-DAY COMPRESSIVE STRENGTH
- MAXIMUM WATER-CEMENT RATIO 0.50
- AIR-CONTENT 5 TO 7%, WITH AIR-ENTRAINMENT
- MAX CONCRETE TEMPERATURE IS 95°
- SLUMP SHALL BE 2 TO 4 INCHES PRIOR TO ADDITION OF SUPERPLASTICIZING ADMIXTURES BEING ADDED, 3 TO 6 INCHES WITHOUT USE OF SUPERPLASTICIZERS.
- SLUMP CAN BE 7.5 INCHES MAX WITH THE ADDITION OF SUPERPLASTICIZING ADMIXTURES.
- CONCRETE ADMIXTURES SHALL MEET ASTM-C260 FOR AIR ENTRAINMENT, AND ASTM C494 TYPE A, D, F OR G FOR WATER-REDUCTION AND SET-RETARDATION AND TYPES C OR E FOR NON-CORROSIVE ACCELERATORS.
- ADMIXTURES SHALL BE INCLUDED IN THE DESIGN MIX. FOLLOW DOSAGES AND RECOMMENDATIONS OF MANUFACTURER.
- THE CONTRACTOR(S) SHALL PROVIDE A DESIGN MIX TO LDG FOR APPROVAL PRIOR TO ORDERING CONCRETE. ALL LOAD TICKETS SHALL BE PROVIDED TO AND APPROVED BY THE INSPECTOR ON SITE AND SHALL REFLECT ALL MATERIALS AND QUANTITIES INCLUDING ADMIXTURES, AMOUNT OF WATER (METERED WATER AND FREE MOISTURE IN THE AGGREGATE), AND TOTAL SIZE OF THE BATCH. THE BATCH TICKET MUST INDICATE THE AMOUNT OF WATER THAT MAY BE ADDED ON-SITE WHILE MAINTAINING THE DESIGN REQUIREMENTS OR NO WATER MAY BE
  - ADDFD.
- 10. CEMENTITIOUS MATERIAL MAY CONTAIN UP TO 20% SLAG IN THE MIX.
- 11. THE CONCRETE MIX MAY CONTAIN TYPE 1L PORTLAND-LIMESTONE CEMENT: NOT TO **EXCEED 10% LIMESTONE CONTENT**

# **PLACEMENT**

- CONCRETE SHALL ONLY BE PLACED IN THE PRESENCE OF AN LDG INSPECTOR.
- PLACEMENT DURING HOT OR COLD WEATHER WILL REQUIRE A WRITTEN PLAN IN ADVANCE DETAILING CONCRETE CONDITIONS, PLACEMENT PROVISIONS, AND A CURING PLAN.
- CONCRETE SHALL NOT BE PLACED UNTIL THE SUBGRADE, FORMS, AND STEEL REINFORCEMENTS HAVE BEEN INSPECTED AND APPROVED BY LDG. NOTIFICATION SHALL BE GIVEN FAR ENOUGH IN ADVANCE TO PROVIDE TIME FOR INSPECTION.
- NO WATER MAY BE ADDED AFTER A SUPERPLASTICIZER.
- CONCRETE SHALL BE CONVEYED FROM THE MIXER TO THE FORMS AS RAPIDLY AS PRACTICAL BY METHODS THAT WILL PREVENT SEGREGATION OF THE AGGREGATES OR LOSS OF MORTAR. CONCRETE SHALL BE PLACED WITHIN 1.5 HOURS AFTER THE INTRODUCTION OF CEMENT TO THE AGGREGATE UNLESS AN APPROVED SET-RETARDING ADMIXTURE IS USED IN THE MIX: DURING PERIODS OF HOT WEATHER, IT MAY BE NECESSARY TO REDUCE THIS TIME.
- CONCRETE SHALL NOT BE DROPPED MORE THAN 5 FEET VERTICALLY. SUPERPLASTICIZED CONCRETE SHALL NOT BE DROPPED MORE THAN 12 FEET VERTICALLY.
- 7. FORMED WALLS SHALL BE PLACED IN 2' LAYERS UNLESS SUPERPLASTICIZER IS USED. IN WHICH CASE THE MAXIMUM LAYER SHALL BE S'. EACH LAYER SHALL BE CONSOLIDATED TO ENSURE A GOOD BOND WITH THE PRECEDING LAYER.
- CONCRETE SHALL BE CONSOLIDATED BY VIBRATING IMMEDIATELY AFTER PLACEMENT AND EXTEND A MINIMUM OF 6" INTO THE PREVIOUSLY CONSOLIDATED LAYER.
- CONCRETE SHALL BE WORKED INTO CORNERS, ANGLES, AND ALL AROUND REINFORCEMENT AND EMBEDDED ITEMS IN A MANNER THAT PREVENTS SEGREGATION OR THE FORMATION OF "HONEYCOMBING"
- 10. VIBRATION SHALL NOT BE USED TO MAKE CONCRETE FLOW.
- 11. IF THE SURFACE OF A PREVIOUSLY PLACED LAYER OF CONCRETE HAS TAKEN A SET TO THE DEGREE THAT IT WILL NOT MIX WITH THE PRECEDING LAYER WHEN VIBRATED, THE CONTRACTOR SHALL DISCONTINUE PLACING CONCRETE AND FORM A CONSTRUCTION JOINT TO AVOID A "COLD JOINT". VINYL WATERSTOP AND FORM MATERIAL SHALL BE ON SITE PRIOR TO STARTING THE PLACEMENT OF ANY CONCRETE.
- 12. THE LANDOWNER HAS THE OPTION OF HAVING GROOVES FLOATED OR CUT INTO THE STRUCTURE FLOOR(S) FOR ADDED TRACTION FOR ANIMALS AND EQUIPMENT. THIS DECISION WILL BE CONVEYED TO THE CONTRACTOR(S) DURING PRICE SOLICITATION

### **CURING**

- CONCRETE SHALL BE ALLOWED TO CURE AT LEAST 24 HOURS PRIOR TO BEGINNING FORM OR REINFORCEMENT PLACEMENT FOR ADJACENT CONSTRUCTION.
- NO EQUIPMENT SHALL BE ALLOWED ON CONCRETE SLABS OR FLOORS UNTIL THE CONCRETE HAS CURED FOR A MINIMUM OF 7 DAYS. THIS INCLUDES ANY MOTORIZED MATERIAL HANDLING EQUIPMENT, PALLETS OF FORMS, ETC. SKID LOADERS USED FOR TRANSPORTING CONCRETE INTO FORMS SHALL NOT BE ALLOWED ON SLABS OR FLOORS FOR A MINIMUM OF 14 DAYS.
- 3. FORMS FOR WALLS SHALL NOT BE REMOVED FOR AT LEAST 24 HOURS AFTER PLACING THE CONCRETE. IF FORMS ARE REMOVED IN LESS THAN 7 DAYS, THE EXPOSED CONCRETE SHALL BE SPRAYED WITH CURING COMPOUND.
- 4. CURING COMPOUND SHALL BE APPLIED IN A UNIFORM LAYER OVER ALL SURFACES REQUIRING PROTECTION AT A RATE AS DESIGNATED BY THE MANUFACTURER. CURING COMPOUND SHALL BE REAPPLIED IF DISTURBED WITHIN 3 HOURS AFTER BEING APPLIED.
- WALLS SHALL BE ALLOWED TO CURE FOR A MINIMUM OF 7 DAYS BEFORE INSTALLING "DRILL SET" POST BRACKET ANCHORS. WALLS SHALL BE ALLOWED TO CURE FOR A MINIMUM OF 3 DAYS BEFORE INSTALLING POSTS IN/ON "WET SET" BRACKETS. ALL WALL TIES. HONEY-COMBING, AND AIR HOLES > 3/4" SHALL BE PARGED WITH NON-SHRINK GROUT.
- 6. RANDOM CRACKING IN THE WALLS AND FLOOR SHALL BE EVALUATED AND DETERMINED IF THE CONCRETE NEEDS TO BE REMOVED OR REPAIRED. REMOVAL AND REPAIR SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND AT NO INCREASE IN COST.
- 7. IF MAJOR REPAIRS ARE REQUIRED, THE CONTRACTOR SHALL PREPARE A WRITTEN REPAIR PLAN WITH ALL MATERIALS AND METHODS CLEARLY STATED AND SHALL BE APPROVED BY THE LDG ENGINEER OF AUTHORITY BEFORE PROCEEDING WITH THE

## **JOINTS**

- BEFORE NEW CONCRETE IS PLACED ON OR AGAINST CONCRETE THAT HAS SET, THE SURFACE OF CONSTRUCTION JOINTS SHALL BE CLEANED OF ALL LAITANCE AND DEBRIS BY HIGH-PRESSURE WATER CUTTING, WASHING AND WIRE-BRUSHING, OR AS APPROVED BY THE ENGINEER. THE SURFACE OF THE IN-PLACE CONCRETE SHALL BE CUT TO EXPOSE CLEAN, SOUND AGGREGATE, BUT NOT SO DEEP TO UNDERCUT THE EDGES OF THE LARGE AGGREGATE. ALL CONSTRUCTION JOINTS SHALL BE WETTED FOR AT LEAST 1-HOUR PRIOR TO NEW PLACEMENT AND STANDING WATER SHALL BE REMOVED.
- SLAB CONTROL JOINTS SHALL BE SAW-CUT AS SOON AS POSSIBLE, BUT NO LATER THAN 24 HOURS AFTER PLACEMENT OF THE CONCRETE, AT THE INTERVALS INDICATED ON THE DRAWINGS. ALL JOINTS SHALL BE WATER TIGHT AND AS SHOWN ON THE DETAIL DRAWINGS. THE SAW-CUTS SHALL BE THOROUGHLY CLEANED AND DRIED SO THE SEALANT AND PRIMER WILL BOND TO THE CONCRETE.
- 3. FOR THE JOINTS IN THE DRAWINGS THAT CALL FOR AN ELASTOMERIC SEALANT, THE SEALANT SHALL MEET THE REQUIREMENTS STATED IN THE CONSTRUCTION SPECIFICATION, INCLUDED IN THIS DESIGN PACKAGE, AND SHALL ALSO MEET THE FOLLOWING: THE SEALANT SHALL BE TYPE S (SINGLE COMPONENET), CLASS 25, AND MEET THE REQUIREMENT FOR TYPE (ABLE TO BE IMMERSED IN LIQUID). SOME SEALANTS REQUIRE A PRIMER TO BE USED BEFORE THE SEALANT IS APPLIED; PRIMERS SHALL BE USED NO MATTER IF THE JOINT IS LOCATED IN A "SUBMERGED" CONDITION OR NOT. IT IS RECOMMENDED THAT THE PRIMER IS SUPPLIED BY THE SAME MANUFACTURER AS THE SEALANT, THIS WILL ENSURE THAT THE SEALANT AND PRIMER ARE COMPATIBLE.

### **TESTING REQUIREMENTS**

1. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT THE CONCRETE MEETS THE DESIGN REQUIREMENTS. THE CONTRACTOR SHALL TEST THE CONCRETE AS NEEDED; SLUMP, AIR ENTRAINMENT, CONCRETE TEMPERATURE, AND CYLINDERS. ALL CONCRÉTE FOR TESTING OR MAKING CYLINDERS SHALL BE TAKEN FROM THE DISCHARGE END OF THE PUMP TRUCK. THE LDG, PACD, OR CONSERVATION DISTRICT INSPECTOR MAY TEST THE CONCRETE AS THEY FEEL THE NEED TO DO SO. THE CONTRACTOR IS NOT TO RELY ON THE INSPECTOR TO PROVIDE THE TESTING SERVICE.



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# TRUSS DESIGN NOTES

- TRUSSES SHALL BE USED FOR THIS ROOF. SHOP DRAWINGS SHALL BE PROVIDED TO THE LDG DESIGN ENGINEER FOR APPROVAL PRIOR TO ORDERING THE TRUSSES AND "PE"(PROFESSIONAL ENGINEER) SEALED SHOP DRAWINGS SHALL BE SUPPLIED BY THE TRUSS PLATE INSTITUTE CERTIFIED MANUFACTURER AT THE TIME OF TRUSS DELIVERY. (TRUSS AND STRINGER CONFIGURATION SHOWN IN THE DRAWINGS IS FOR ILLUSTRATION PURPOSES ONLY) LDG DOES NOT DESIGN ROOF TRUSSES.
- \*MAKE THE TRUSS DESIGNER AWARE OF KNEE BRACING BEING USED.
- ALL NAILS SHALL HAVE FULL HEADS; CLIPPED HEADS ARE NOT ACCEPTABLE.
- ALL NAILS AND BOLTS USED WITH PRESSURE TREATED WOOD SHALL BE HOT -- DIP GALVANIZED NAILS THAT MEET THE MINIMUM GALVANIZED COATING REQUIREMENTS FOR THE MOST RESTRICTIVE WOOD PRESERVATIVE TREATMENT METHOD. (I.E. CCA TREATED WOOD REQUIRES A MINIMUM COATING RATING OF G--90 HOWEVER ACQ TREATED WOOD REQUIRES A COATING RATING OF G--185. WHEN THE WOOD TYPES ARE MIXED, USE THE G--185 CONNECTORS. CONSULT WITH INDIVIDUAL FASTENER, HARDWARE MANUFACTURER FOR RECOMMENDATIONS)
- CAUTION: NEW WOOD PRESERVATIVE TREATMENT METHODS REQUIRE SPECIAL FASTENERS AND CONNECTORS. ALL PLATES AND FASTENERS USED WITH ACQ, CBA OR CA TREATMENT FORMULAS MUST CONFORM TO ASTM STANDARDS; ASTM A153 FOR HOT-- DIP FASTENERS, AND A653 FOR HOT -- DIP CONNECTOR AND SHEET PRODUCTS. THIS CHANGE INCREASES THE GALVANIZED COATING REQUIREMENTS TO A DESIGNATION OF G--185.STAINLESS STEEL FASTENERS AND CONNECTIONS MAY BE USED IN PLACE OF HOT-- DIP GALVANIZED PRODUCTS.
- NAILS FOR GENERAL FRAMING CAN BE COMMON, FULL HEAD SIZE 16D OR LARGER, SMOOTH NAILS. GENERAL FRAMING INCLUDES PURLINS, DIAGONAL BRACES, LATERAL
- BOLTS, SCREWS, OR METAL PLATE CONNECTORS MAY BE USED INSTEAD OF NAILS. SUCH SUBSTITUTIONS SHALL PROVIDE A CONNECTION OF EQUAL OR GREATER STRENGTH AND DURABILITY, ACCORDING TO THE NOTION AT FOREST PRODUCTS ASSOCIATION'S (NFPA) NATION AT DESIGN SPECIFICATION. ALTERNATE CONNECTORS MUST BE APPROVED BY THE DESIGN ENGINEER.
- 6. ALL WOOD IN CONTACT WITH THE GROUND OR MANURE SHALL BE PRESSURE TREATED AS PER AMERICAN WOOD PRESERVER'S ASSOCIATION STANDARD (POSTS SHALL BE TREATED TO 0.6 #/CU.FT. AND ALL OTHER WOOD SHALL BE TREATED TO 0. 4 #/CU.FT.)
- 7. ALL STRUCTURAL MEMBERS WHICH INCLUDES: ALL WYE AND KNEE BRACING, BÉARING BLOCKS, TRUSS SUPPORT BLOCKS, AND GIRDERS/HEADERS; (EXCLUDING MICROLLAM GIRDERS/HEADERS) SHALL BE SOUTHERN YELLOW PINE OR DOUGLAS FIR--LURCH NO. 2 GRADE
- (SURFACE DRY, USED AT 19% MAXIMUM MOISTURE CONTENT). ALL SECONDARY MEMBERS SUCH AS PERMANENT OR CONTINUOUS BRACING SHALL BE (SYP) SOUTHERN PINE NO. 3, (SPF) SPRUCE-- PINE-- FIR NO. 2 OR BETTER. PURLINS SHALL BE SYP NO. 2, SPF NO. 2, OR BETTER IF SPACED AT 2' CENTERS. PURLINS SHALL BE SYP NO. 3 OR BETTER IF SPACED AT 1.5' CENTERS.
- 8. POSTS ARE TO BE 4-- PLY & 5-- PLY 2X8 GLU LAMINATED (AS SHOWN IN THE DRAWING) & PRESSURE TREATED, #2 GRADE SYP (SOUTHERN YELLOW PINE). POSTS ARE TO BE FULLY PRESSURE TREATED THE ENTIRE HEIGHT.
- 9. GALVANIZED ANGLE IRON (1/4" THICK X 3" WIDE BOTH WAYS) CAN BE INSTALLED THE CORNERS OF THE POSTS AT ENTRANCE LOCATIONS. OTHER MEANS OF POST
- PROTECTION MAY BE USED IF APPROVED BY THE DESIGN ENGINEER.

  10. KNEE AND WYE BRACING ARE REQUIRED FOR THE POSTS AND GIRDERS AS SHOWN. NO WYE BRACING SHALL BE INSTALLED ON THE "INSIDE" OF THE ENTRANCE LOCATIONS.
- 11. PERMANENT CONTINUOUS LATERAL BRACING IS REQUIRED, ACCORDING TO THE TRUSS MFG DRAWINGS. CONTINUOUS LATERAL BRACING MUST BE INSTALLED WITH STAGGERED SIDE BY SIDE OVERLAP CONNECTIONS (NO BUTT TO BUTT CONNECTIONS). THE ENDS OF THE BRACES MUST EXTEND FULLY PAST THE TRUSS AND ALLOW A 2-NAIL CONNECTION WITHOUT USING TOENAILS.
- 12. PERMANENT DIAGONAL BRACING IS REQUIRED AT EACH END OF THE BUILDING AND AT INTERVALS NOT TO EXCEED WHAT IS SHOWN IN THE DRAWINGS. ALL BRACING SHALL BE INSTALLED AS PER THE TRUSS PLATE INSTITUTE BCSI-B3 AND THE DETAILED DRAWING.
- 13. ROOFING MATERIAL SHALL BE STEEL OR ALUMINUM. STEEL SHALL BE: GALVANIZED STEEL, PAINTED GALVANIZED STEEL, OR PAINTED STEEL. TYPE OF ROOFING TO BE DISCUSSED WITH LANDOWNER PRIOR TO BID SOLICITATION. STEEL ROOFING MATERIAL SHALL BE 29 GAUGE MINIMUM. ALUMINUM ROOFING MATERIAL SHALL HAVE A MINIMUM NOMINAL THICKNESS OF 0.018 INCHES. GALVALUME ROOFING IS NOT PERMITTED FOR USE.
- 14. ROOF FASTENERS SHALL BE A COMBINATION OF ZINC COATED STEEL AND NEOPRENE WASHER. DOUBLE STITCH THE SEEMS OF THE ROOF EDGES. TYPICAL STEEL ROOF SHALL FASTENERS ON A 9" SPACING ON THE PURLINS 24" ON CENTER.
- 15. END TRUSSES SHALL BE FACED WITH ROOFING MATERIAL, AS SPECIFIED ABOVE. THIS SHALL BE DISCUSSED WITH THE LANDOWNER PRIOR TO BID SOLICITATION.
- 16. VENTILATION SHALL BE PROVIDED BY AN "OVERSHOT" TOP CHORD OF THE TRUSS. AS SHOWN ON THE DRAWINGS: A MINIMUM OF 14" OPENING IS REQUIRED FOR THIS STRUCTURE.

- 17. BIRD NETTING IS REQUIRED ON THE BOTTOM CHORD OF THE TRUSS.
- 18. GIRDER REQUIREMENTS:
  - 1.75" x 9.25" LVL'S: MOMENT RATING=6,271 FT-LBS, FB=2,900 PSI, FV=320 PSI,  $E=2.0x10^6$
  - **HEADER REQUIREMENTS:**
- 7" x 18" PSL: MOMENT RATING=87,235 FT-LBS, FB=2,900 PSI, FV=290 PSI, E=2.0X106 19. THE ROOF WAS DESIGNED TO CARRY A COMBINED LOADING OF 40 PSF. ACCORDING TO ASCE-7 (MOST CONSERVATIVE COMBINED LOAD FORMULA). ON THE ENTIRE ROOF SURFACE. THE ROOF WAS ALSO DESIGNED FOR A UNIFORM UPLIFT OF 14.5 PSF UNDER
- THE ENTIRE ROOF. THIS ROOF IS DESIGNED FOR (2) "EN CLOSED" SIDES; MAJOR STRUCTURAL CHANGES MAY BE NEEDED IF THE SIDES THAT ARE LABELED AS "OPEN" ARE ENCLOSED. CONSULT WITH THE DESIGN ENGINEER IF ADDITIONAL ENCLOSED SIDES ARE BEING CONSIDERED.

# **DESIGN CRITERIA**

THE FOLLOWING DESIGN CRITERIA ARE SUITABLE FOR AGRICULTURAL BUILDINGS.

• STRUCTURAL CODES: 2018 IBC, ASCE 7-16, NDS 2018, ACI 318-14 ASAE EP486.3, ASAE EP484

BUILDING RISK CATEGORY:

DEAD LOADS:

TOP CHORD OF ROOF TRUSS: 5 PSF BOTTOM CHORD OF ROOF TRUSS: 5 PSF TOTAL ROOF LOAD: 10 PSF

SNOW LOADS

GROUND SNOW LOAD, PG: 35 PSF SNOW RISK FACTOR: 1.0 THERMAL FACTOR, CT: 1.2 EXPOSURE FACTOR, CE: 1.0 SLOPED ROOF FACTOR, CS: •• 1.0 •• FLAT ROOF SNOW LOAD, PF: 26.95 SLOPED ROOF SNOW LOAD, PS: 24 PSF UNBALANCED WINDWARD SNOW LOAD: 7 PSF

UNBALANCED LEEWARD SNOW LOAD:

45 PSF 10 FT FROM THE RIDGE, THEN 24 PSF 60 FT WIDE BUILDINGS: NOT EVALUATED; REQUIRES ADDITIONAL DESIGN SNOW DRIFT LOAD: •• SLIDING SNOW LOAD: NOT EVALUATED: REQUIRES ADDITIONAL DESIGN

• ROOF LIVE (CONSTRUCTION) LOADS: 30 PSF

BASIC (ULTIMATE) WIND SPEED: 105 MI/H EXPOSURE CATEGORY: C BUILDING MID-HEIGHT: 25 (OR LESS) •• WIND DIRECTIONALITY FACTOR, KD: •• 0.85 TOPOGRAPHIC FACTOR, KZT: 1.0

•• VELOCITY PRESSURE, QH: BUILDING ENCLOSURE CATEGORY:

SHEATHED SIDEWALLS: PARTIALLY ENCLOSED (ALSO ENCLOSED,

22.7 PSF

PARTIALLY OPEN)

OPEN (ALSO PARTIALLY OPEN) OPEN SIDEWALLS:

SEISMIC LOADS: NOT EVALUATED

• RAIN AND ICE LOADS: NOT EVALUATED

STORAGE LOADS (FILL):

DRY BULK DENSITY: 70 PCF 32° ANGLE OF INTERNAL FRICTION: ANGLE OF FILL LINE: ••

•• KA: 0.85 (RANKINE)

•• EFD 60 PSF/FT (APPLIED AT 32°)

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		REVIEW SET	COMMENTS

685 WATERTON ROAD SHICKSHINNY,PA 18655 Stevens Eric

NOTES

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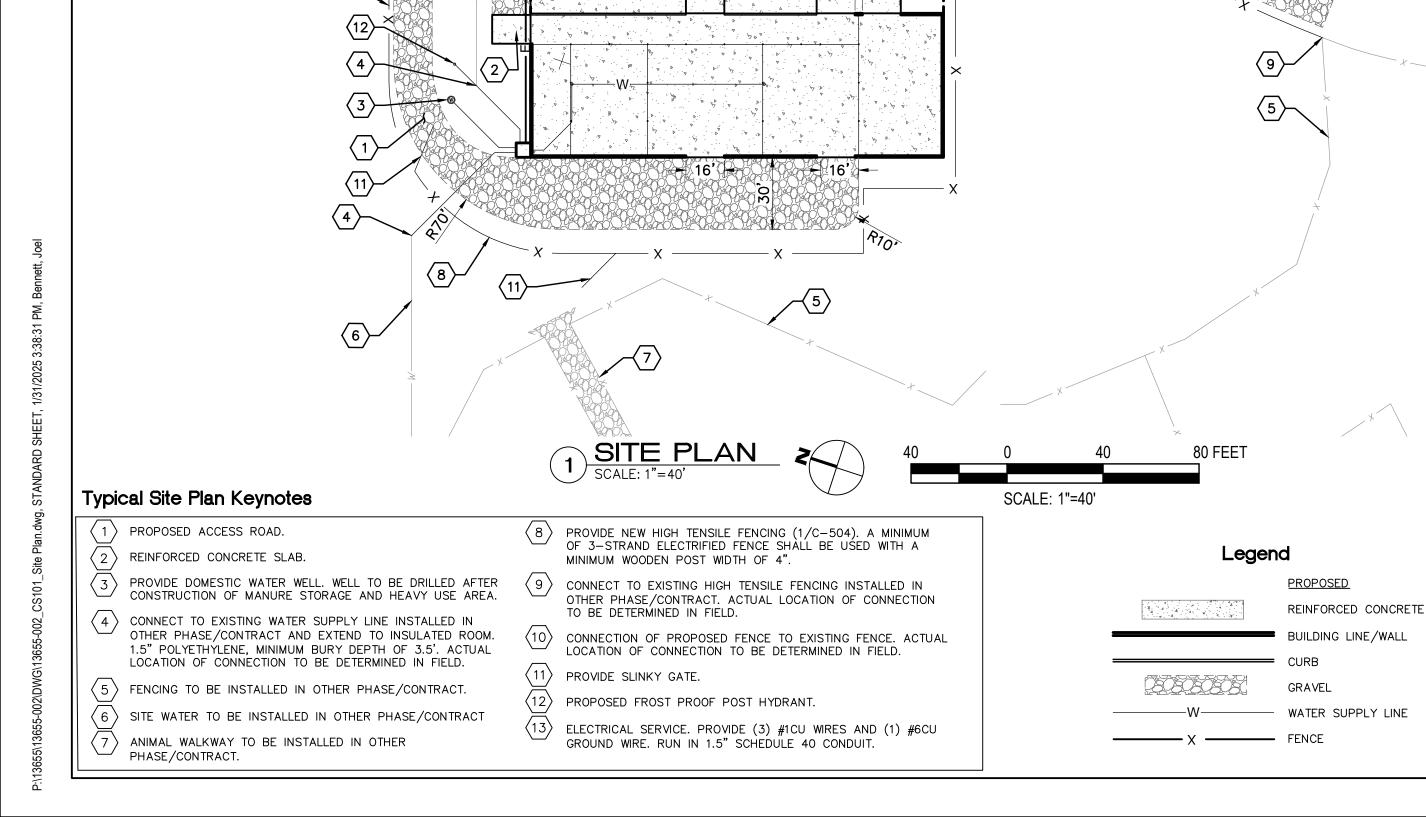
**FARM PROJECT** 685 WATERTON ROAD SHICKSHINNY,PA 18655 GENERAL **STEVENS** 

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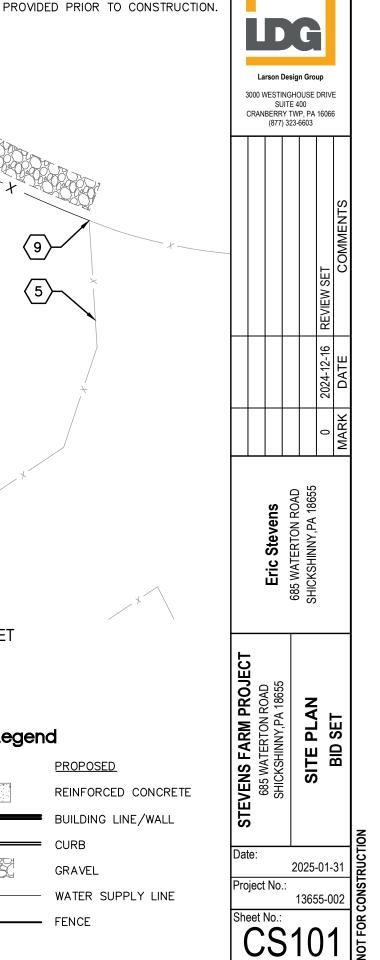
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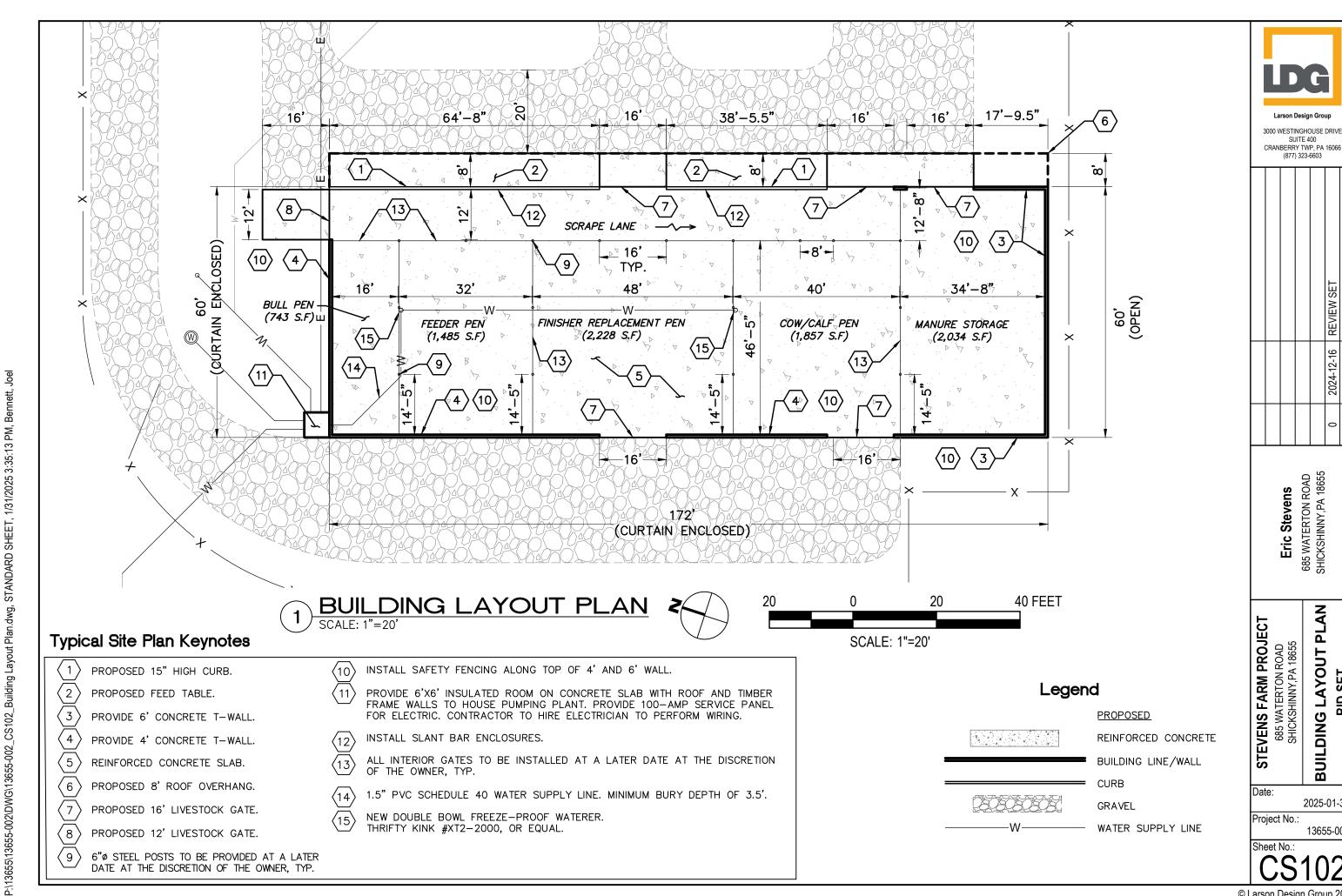


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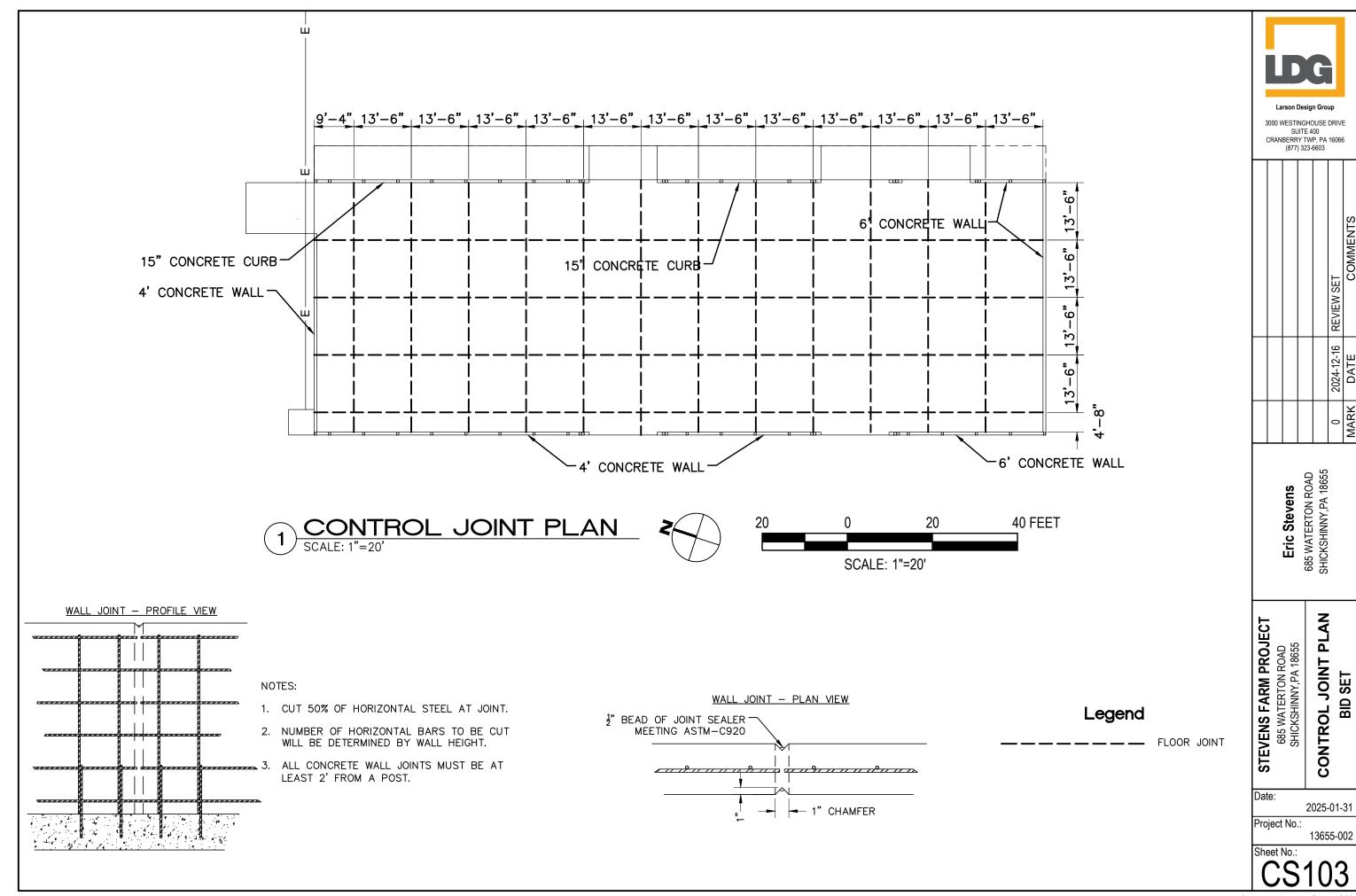
LAYOUT

BUILDING

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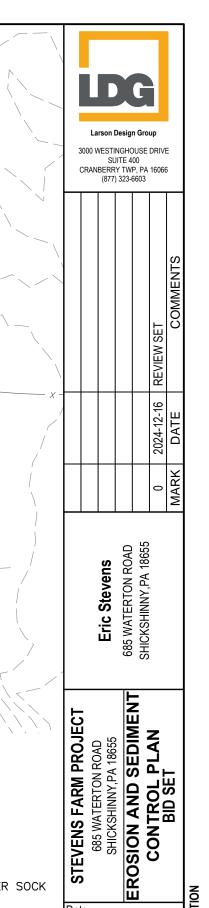
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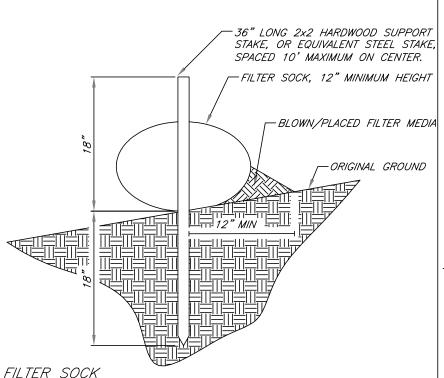


Legend

12" COMPOST FILTER SOCK

**300 FEET** 

2025-01-31 Project No.:

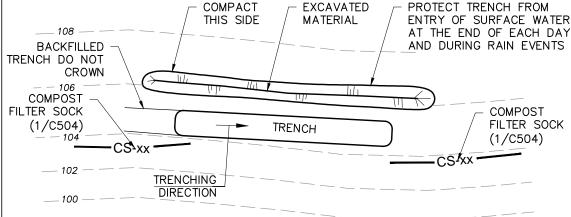


- AREAS OF THE CONSTRUCTION SITE.
- TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
- FILTER SOCK SHALL BE PLACED AT LEVEL EXISTING GRADE. BOTH ENDS OF THE SOCK SHALL BE EXTENDED AT LEAST 8' UP SLOPE AT 45 DEGREES TO THE MAIN FENCE ALIGNMENT.
- STAKES MAY BE INSTALLED IMMEDIATELY DOWN SLOPE OF THE SOCK IF SO SPECIFIED BY THE MANUFACTURER.
- ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT REACHES HALF THE ABOVE GROUND HEIGHT OF THE SOCK.
- SOCKS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT. DAMAGED SOCKS SHALL BE REPAIRED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS OR REPLACED WITHIN 24 HOURS
- BIODEGRADABLE FILTER SOCKS SHALL BE REPLACED AFTER 6 MONTHS; PHOTODEGRADABLE SOCKS AFTER 1 YEAR. POLYPROPYLENE SOCKS SHALL BE REPLACED ACCORDING TO THE
- MANUFACTURER'S RECOMMENDATIONS. UPON STABILIZATION OF THE AREA TRIBUTARY TO THE SOCK, STAKES SHALL BE REMOVED. THE SOCK MAY BE LEFT IN PLACE AND VEGETATED OR REMOVED. IN THE LATTER CASE, THE MESH SHALL BE CUT OPEN AND THE MULCH SPREAD AS A SOIL
- ANY SECTION OF SILT FENCE WHICH HAS BEEN UNDERMINED OR TOPPED SHALL BE IMMEDIATELY REPLACED WITH A ROCK FILTER



## Seeding Preparation

- 1. When grading is finished, apply lime and fertilizer in accordance with soil test recommendations.
- 2. If soil test results are not available, apply 4 ton per acre of agricultural grade limestone and fertilize at the rate of 1,000 lbs. Of 10-20-20 or equivalent per acre.
- 3. Lime and one-half (1/2) the amount of the fertilizer shall be incorporated 4 to 6 inches into the soil.
- 4. Work area with chisel plow or similar type equipment, making sure lime and fertilizer are worked well into the soil.
- 5. Follow with the balance of fertilizer and seed.



# PLACEMENT OF EXCAVATED TRENCH MATERIAL SCALE: N.T.S

CONSTRUCTION PARALLEL

TO SLOPE

# Seeding Recommendation

6. The seed mixture shall be the following or similar if approved by the NRCS representative.

Nurse Crop (required with every permanent seed application):

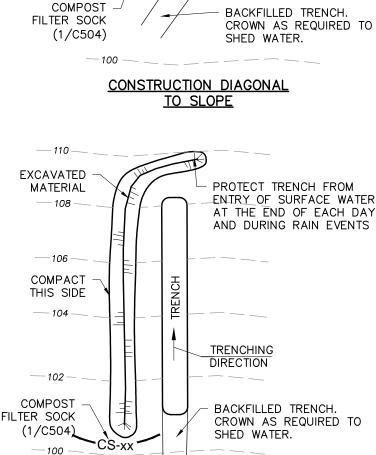
64 lbs/acre PLS Oats 90 lbs/acre PLS 40 lbs/acre PLS Wheat Annual Rye Permanent Stabilization: 40 lbs/acre PLS Perennial Rye PLUS

Tall Fescue 80 lbs/acre PLS NOTE: This mixture is suitable for frequent mowing.

Do not cut shorter than 4". PLS means pure, live, seed. PLS is the product of the percentage of pure seed times percentage germination divided by 100. For example, to secure the actual planting rate for switchgrass, divide 12 lbs PLS by the PLS percentage shown on the seed tag. Thus, if the PLS content of a given seed lot is 35%, divide by .35 to obtain 34.4 lbs of seed, the amount of seed required to plant 1 acre. If partial completion of any part of the project is accomplished, and this area will be disturbed again BUT not for a period of 20 days or more, those areas must be seeded with a TEMPORARY cover-seeding.

Temporary Seed and mulch will be applied at the following fates:

40 lbs/Acre Annual Ryegrass Winter Rye 3 Bu/Acre Winter Wheat 3 Bu/Acre Spring Oats 3 Bu/Acre



PROTECT TRENCH FROM ENTRY OF SURFACE WATER

AT THE END OF EACH DAY

AND DURING RAIN EVENTS

**TRENCHING** 

DIRECTION

— 110 —

**EXCAVATED** 

COMPACT

THIS SIDE

-102

MATERIAL

# Planting Recommendation

Seed can be applied with a drill or broadcast seeder.

Band seeding is not permitted. If broadcast, harrow or disk lightly to cover seed. Roll with cultipacker or similar roller in same direction as seeding. (Double drilling gives better distribution of seeding and helps to spread the water while plants are small. Drill first lengthwise and then crosswise (in a zig-zag pattern). Optimum planting time is early spring or mid summer

CONSTRUCTION PERPENDICULAR

TO SLOPE

7. As soon as seeding is finished, mulch with 3 Tons/Acre of hay or straw, making a layer 1 to 1.5 inches deep. Set disk straight and go over mulch to press straw into the soil. Tackifiers can also be used for anchoring mulch.



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685 WATERTON ROAD SHICKSHINNY,PA 18655

**STEVENS** 

Date:

Project No.:

Sheet No.:

**DETAILS** 

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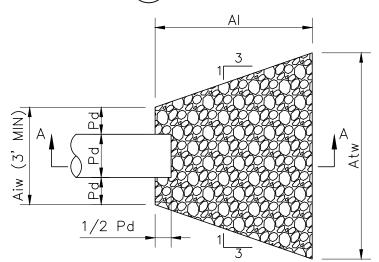
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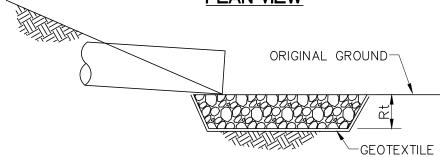
- 1. Geotextile shall be as per General Notes drawing. Placement shall provide a one-foot (1') overlap between adjacent panels.
- 2. Stone depth shall be measured after compaction.
- 3. All stone shall be compacted with a smooth drum, vibratory roller.

GEOTEXTILE

# **ACCESS ROAD**



# **PLAN VIEW**

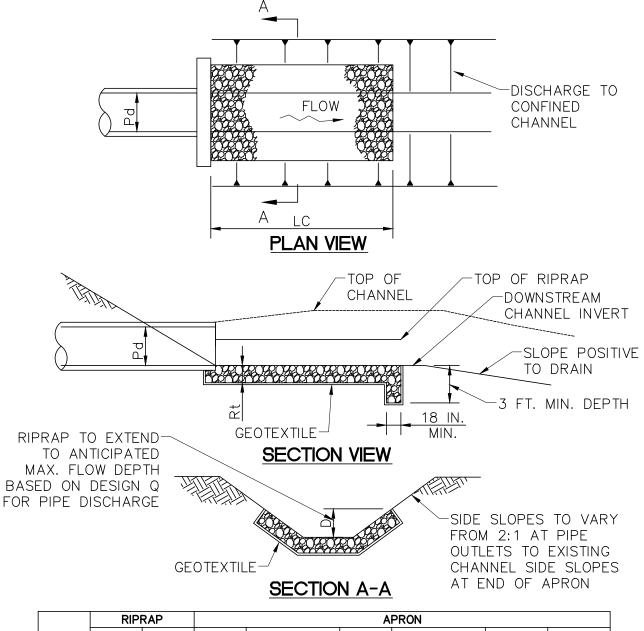


# SECTION A-A

PIPE DIA	RIPI	RAP	APRON			
PIPE DIA Pd (IN)	SIZE R		LENGTH AI, (FT)	INITIAL WIDTH Aiw, (FT)	TERMINAL WIDTH Atw, (FT)	
4" & 6"	4	12	5	3	6.33	

- ALL APRONS SHALL BE CONSTRUCTED TO THE DIMENSIONS SHOWN. TERMINAL WIDTHS SHALL BE ADJUSTED AS NECESSARY TO MATCH RECEIVING CHANNELS.
- ALL APRONS SHALL BE INSPECTED AND AFTER EACH RUNOFF EVENT. DISPLACED RIP-RAP WITHIN THE APRON SHALL BE REPLACED IMMEDIATELY.
- EXTEND RIP-RAP ON BACK SIDE OF APRON TO AT LEAST 1/2 DEPTH OF PIPE ON BOTH SIDES TO PREVENT SCOUR AROUND THE PIPE.

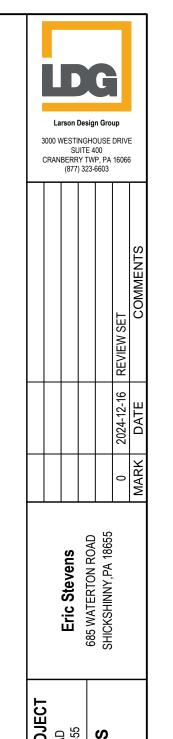
# RIP RAP APRON AT PIPE OUTLET NO FLARED ENDWALL



	RIP	RAP			Α	PRON		
PIPE DIA Pd (IN)	SIZE R	THICK. Rt (IN)	LENGTH LC (FT)	INITIAL BOTTOM WIDTH (AT OUTLET) (FT)	END WIDTH (FT)	INITIAL TOP WDTH (AT OUTLET) (FT)	END TOP WIDTH (FT)	SIDE SLOPES H: V
36	7	30	20	3	3	9	9	3:1

- 1. ALL APRONS SHALL BE CONSTRUCTED TO THE DIMENSIONS SHOWN. TERMINAL WIDTHS SHALL BE ADJUSTED AS NECESSARY TO MATCH RECEIVING CHANNELS.
- 2. ALL APRONS SHALL BE INSPECTED AFTER EACH RUNOFF EVENT. DISPLACED RIP-RAP WITHIN THE APRON SHALL BE REPLACED IMMEDIATELY.

# RIP RAP APRON AT PIPE OUTLET TO CHANNEL SCALE: N.T.S



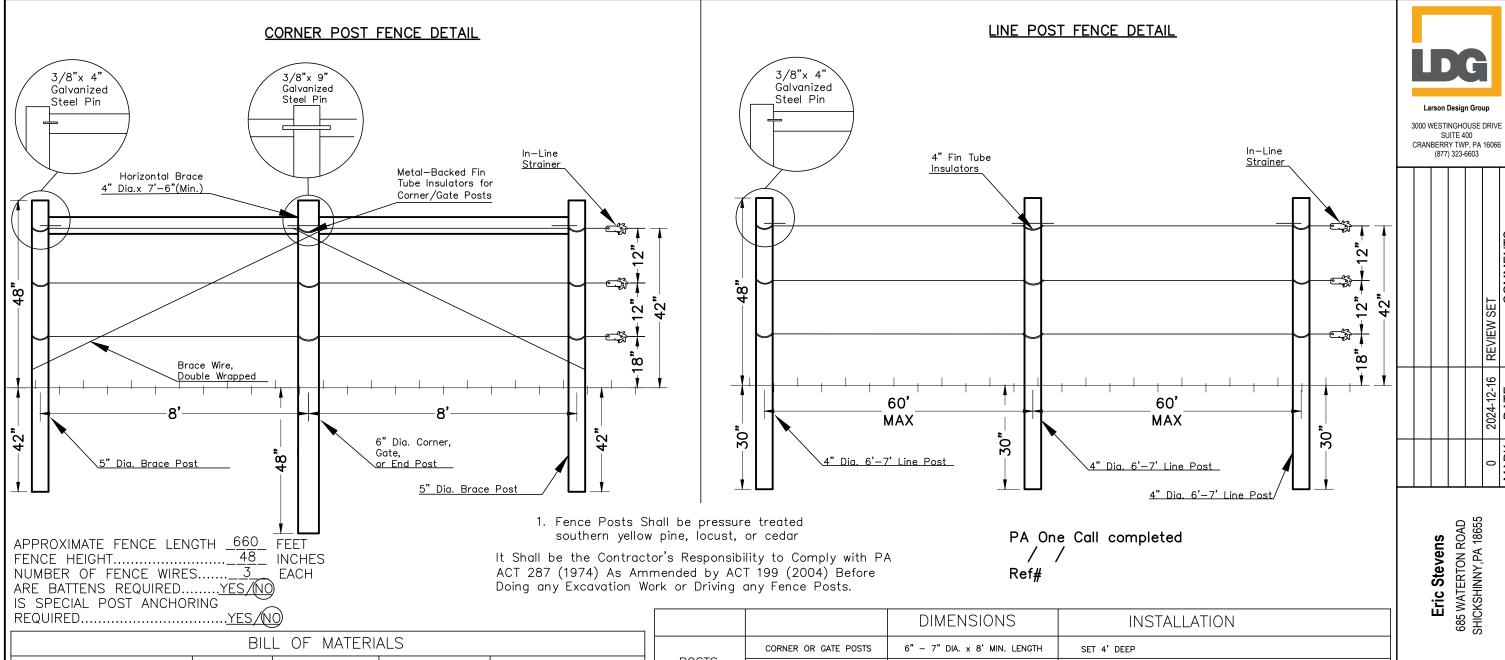
STEVENS FARM PROJECT SITE DETAIL

Date:

2025-01-31 Project No.:

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BILL OF MATERIALS				
ITEM	UNIT	DIA./SIZE	LENGTH	QUANTITY
CORNER, GATE, OR END POST	EACH	6" MIN.	8'-8 1/2'	9 est.
BRACE POSTS	EACH	5" MIN.	7 1/2'-8'	16 est.
LINE POSTS	EACH	4" MIN.	6 1/2' MIN.	12 est.
HORIZONTAL BRACE	EACH	4" MIN.	7 1/2'-8'	16 est.
TENSION SPRINGS	EACH	_	_	3 est.
GALV. HIGH-TENSILE WIRE	LIN. FT.	_	_	2,000 est.
ELECTRIC FENCE CHARGER	EACH	_	-	PER MANUFACTURER REQMTS.
GROUND RODS	EACH		3 1/2' MIN.	PER MANUFACTURER REQMTS.
LIGHTNING ARRESTORS	EACH	_	-	1 PER CHARGER CONNECTION
SLINKY GATES	EACH	-	-	2
LIVESTOCK GATES (12')	EACH	-	12'	0
LIVESTOCK GATES (10')	EACH	-	10'	0
ELECTRIC FENCE WARNING SIGNS	EACH	_	_	1 est.

_			DIMENSIONS	INSTALLATION	Ш 130
		CORNER OR GATE POSTS	6" – 7" DIA. x 8' MIN. LENGTH	SET 4' DEEP	
	POSTS	LINE POSTS	4" DIA. x 6 1/2' MIN. LENGTH	SET 2 1/2' DEEP, SPACE 50' OR LESS	
		BRACE POSTS	5"-6" DIA. x 7 1/2' - 8' LONG	SET 3 1/2' - 4' DEEP	PROJECT ROAD A 18655
		HORIZONTAL BRACE	4"-5" DIA. x 7 1/2' - 8' LONG	PLACE NEAR POST TOPS	AP &B 655
	BRACING	BRACE POST PINS	3/8" x 9" & 3/8" x 4" GALV. STEEL PINS		PR   N RO.
		BRACE WIRE	12 1/2 GAUGE HIGH-TENSILE WIRE, CLASS 3 GALVANIZED	HORIZONTAL BRACE DOUBLE-WRAP, TIGHTEN WITH 1 1/2" x 2" x 2' HARDWOOD TWIST ROD	/ENS FARM PROJ 885 WATERTON ROAD SHICKSHINNY,PA 18655
		TYPE:	12 1/2 GAUGE HIGH—TENSILE, CLAS STRENGTH	S 3, GALVANIZED, MINIMUM 200,000 PSI BREAKING	:NS
	WIRE	SPACING: 3 Strand	FROM GROUND TO BOTTOM WIRE FROM BOTTOM WIRE TO SECOND W FROM SECOND WIRE TO THIRD WIRI		STEVENS 685 W/ SHICKS
	WIIVE	TENSION:	250 — 300 POUNDS EACH WIRE. TEINDICATOR SPRING ON AT LEAST ONE	NSION WITH IN-LINE STRAINERS. INSTALL A TENSION WIRE TO GAUGE WIRE TENSION.	Date:
4		FASTENING:	AT GATE, CORNER, AND END BRACES OR WIRE ANCHOR THROUGH POSTS.	S, USE APPROPRIATE KNOTS OR CRIMPING SLEEVES STAPLE WIRES TO POSTS WITH 1 3/4" 9 GAUGE	
			GALVANIZED STAPLES WITH SLASH CU	JT POINTS. DO NOT DRIVE STAPLES TIGHT IN LINE POST SPLITS. DRIVE INTO POST AT DOWNWARD	Project No.:
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3 STRAND HIGH TENSILE ELECTRIC FENCE SCALE: N.T.S

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SITE DETAILS

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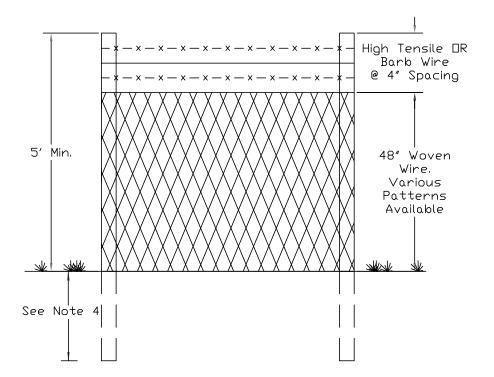
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# SAFETY FENCE OPTIONS

Adjacent to Waste Storage Facilities, Waste Transfer Systems, or Heavy Use Areas with drops exceeding 3.5'



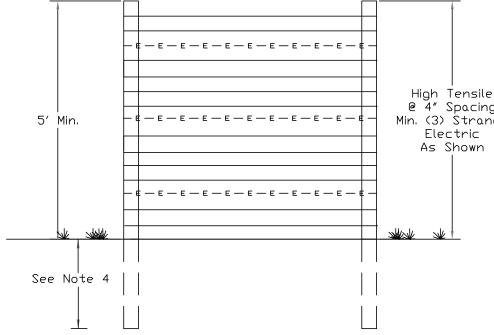
### FOR OPTIONS #1 AND #2:

Woven Wire, 12.5 gauge min. openings not to exceed 6"x6". (welded wire not allowed)

PLUS one of the following:

(1) Barbed wire @ 4" Max. Spacing to Achieve Minimum Fence Height.

(2) High Tensile Wire @ 4" Max. Spacing to Achieve Minimum Fence Height. If using high tensile, at least one strand must be electrified and if more than 2 stands are used then every other strand must be electrified.



# SAFETY FENCE OPTION #1 SCALE: N.T.S

# CONSTRUCTION NOTES (APPLY TO BOTH OPTIONS):

- 1. Gates shall be commercially available metal model approved by NRCS representative prior to installation. Spacing in gate members may not exceed 4" vertically DR individual openings should not exceed 6" x 6". This may be accomplished by attaching woven wire to a standard gate.
- 2. Any woven wire fence types shall be 12-1/2 gauge min, and galvanized.
- 3. High tensile or barbed wire shall be 12-1/2 gauge. Min. 180,000 psi for electrified wire. Min. 200,000 psi for non-electrified wire.
- 4. Line Posts: 4" min. diameter, and 21/2ft min. embedment. Corner/Gate Posts: 6" min. diameter, 3½ft min, embedment,

- 5. Maximum line post spacing is 8' o-c.
- 6. Fence must be grounded accordingly.
- 7. Two-stage latches must be installed at all access points. Two stage latches require (2) actions prior to opening the gate.
- 8. Fence posts must be either Locust or Pressure-Treated Soft Wood.
- 9. Must be in accordance with Township and Local ordinances.
- 10. See Specification 382 Exhibit 3 for additional requirements for bracing and materials.

11. Warning signs required on fencing around manure storages and/or around electric fences as specified in Additional Conditions for project.

NOTE: CHAIN LINK FENCE IS ALSO AN ACCEPTABLE SAFETY FENCE OPTION.

SAFETY FENCE OPTION #2
SCALE: N.T.S

•	- E - E - E - E - E - E - E - E - E - E	
5′ Min. !	- E - E - E - E - E - E - E - E - E - E	High Tensile  @ 4" Spacing  = # Min. (3) Strands  Electric
		As Shown
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See Note 4		

685 WATERTON ROAD SHICKSHINNY,PA 18655 SITE DETAILS SET 읆 2025-01-31

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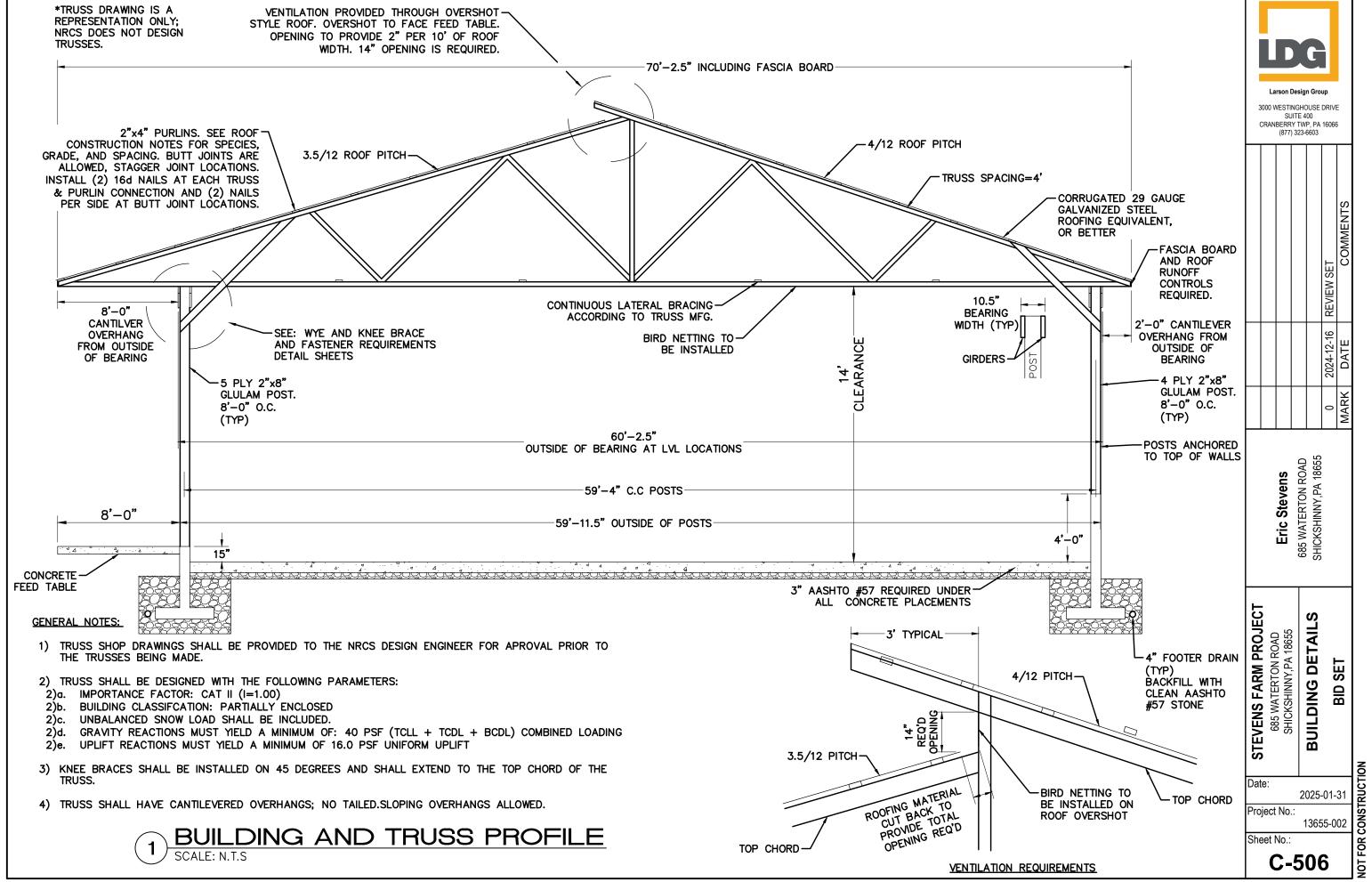
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STEVENS FARM PROJECT

Project No.:

Sheet No.:

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**BUILDING DETAILS** 

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STEVENS FARM PROJECT

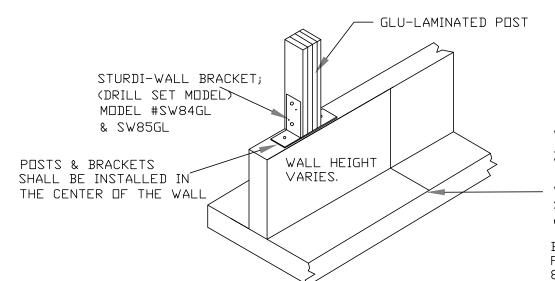
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2025-01-31 Project No.:

Sheet No.:

C-508

13655-002



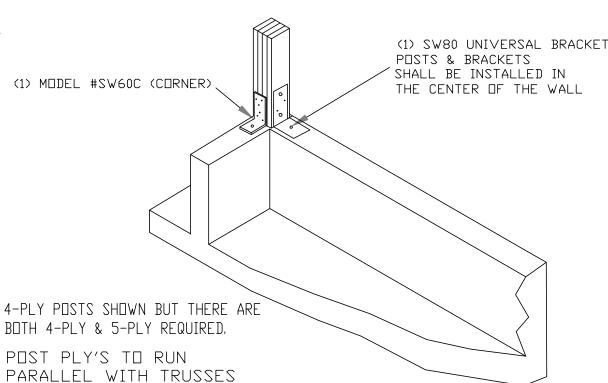
WALLS SHALL BE ALLOWED TO CURE FOR A MINIMUM OF 7 DAYS BEFORE INSTALLING SCREW ANCHORS ALL ANCHORS INTO CONCRETE SHALL BE SCREW -TYPE, OR EPOXY; NO EXPANSION OR WEDGE ANCHORS SHALL BE USED.

POST PLY'S TO BE PARALLEL TO THE TRUSSES.

WALL/FLOOR CONTROL JOINTS MAY LINE UP WITH EACH OTHER.

WALL CONTROL JOINTS SHALL NOT BE INSTALLED AT POST LOCATIONS.

BRACKETS CAN BE PURCHASED FROM: PERMA COLUMN EAST LLC 888-699-8875

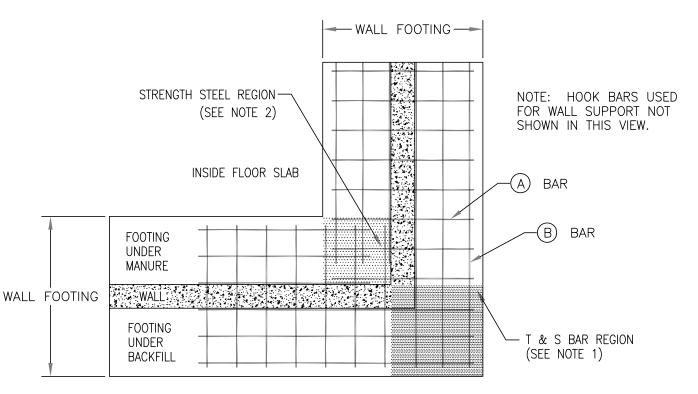


# POST ON WALL SCALE: N.T.S

ADAPTED AND MODIFIED FROM STANDARD DRAWING # PA-023

### NOTES FOR FOOTING STEEL PLACEMENT

- 1.) FOOTING TEMPERATURE AND SHRINKAGE STEEL (T&S) TO BE EXTENDED INTO THIS REGION FROM BOTH SIDES OF CORNER, REGION IS OUTSIDE EXTENSION OF WALLS INCLUDING WALL THICKNESS.
- 2.) STRENGTH STEEL IS EXTENDED INTO THIS REGION FROM BOTH SIDES OF CORNER. REGION IS INSIDE EXTENSION OF THE WALLS. FOOTING SLAB T&S STEEL OUTSIDE THE CORNER REGION TO LAP SPLICE WITH THE STRENGTH STEEL 16 INCHES.
- 3.) IN BOTH CORNER REGIONS, STRENGTH STEEL AND T&S STEEL WILL REQUIRE SWITCHING POSITIONS FROM TOP TO BOTTOM AND VICE VERSA.

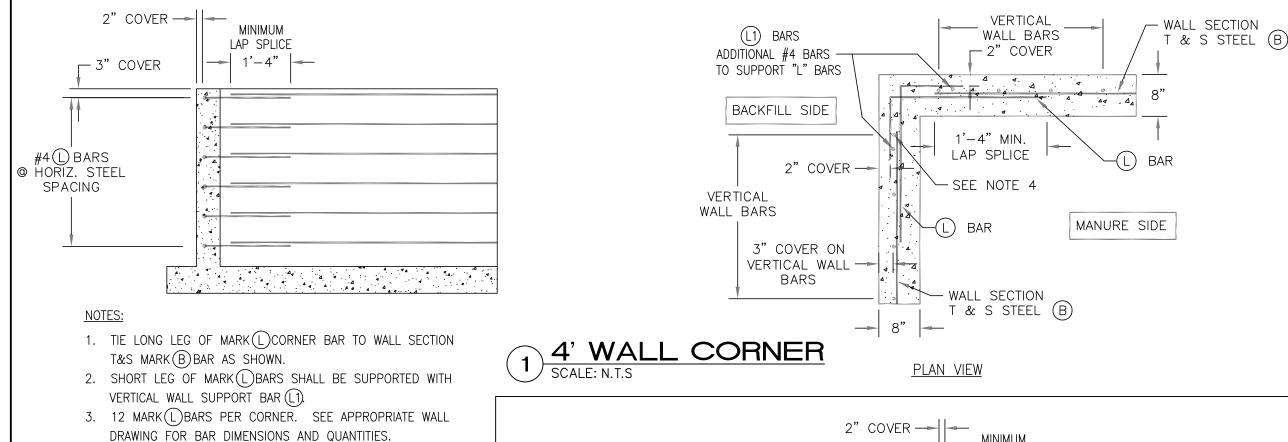


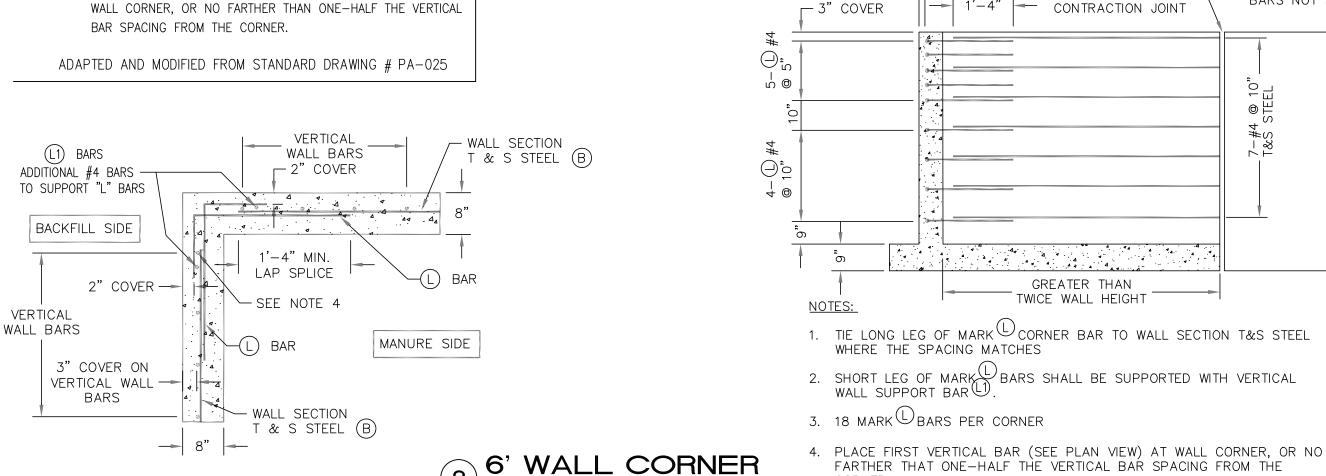
SLAB FOOTING CORNER
SCALE: N.T.S



4. PLACE FIRST VERTICAL BAR (SEE PLAN VIEW) AT

PLAN VIEW

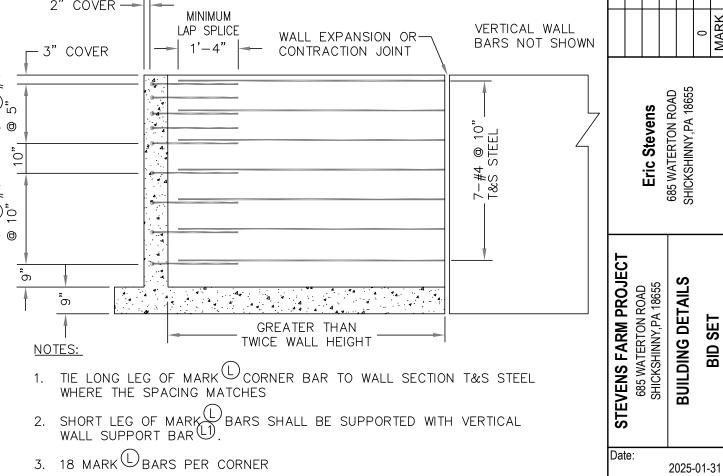




SCALE: N.T.S

CORNER

ADAPTED AND MODIFIED FROM STANDARD DRAWING # PA-027A



C-509

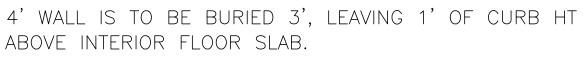
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Project No.:

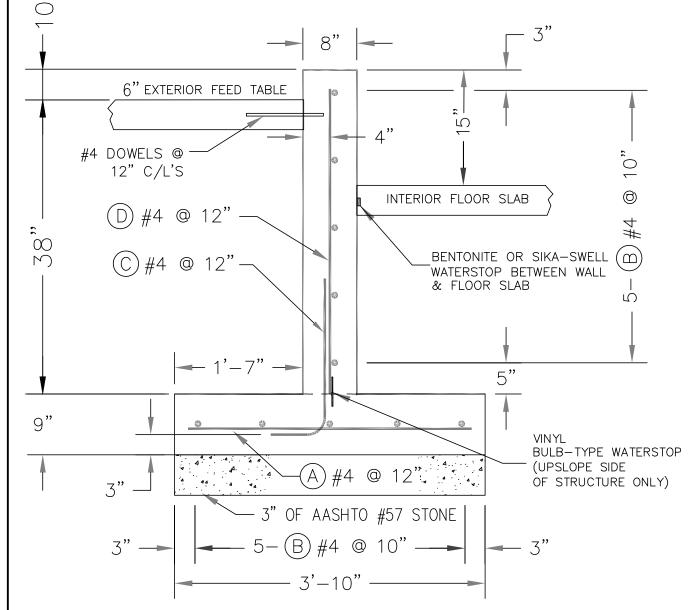
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Larson Design Group

3000 WESTINGHOUSE DRIVE SUITE 400 CRANBERRY TWP, PA 16066 (877) 323-6603



ROOF SUPPORT POSTS TO BE ANCHORED TO TOP OF WALL



# ESTIMATED QUANTITIES

CONCRETE (0.21 CU.YDS./LIN.FT.) \_\_\_\_\_ CU. YDS. (20.67 FT./LIN. FT.) \_\_\_\_\_ FT. STEEL (35.0 FT./CORNER) \_\_\_\_\_ FT. STEEL

- CONCRETE SHALL MEET PA 313 OR 561 SPECIFICATION REQUIREMENTS.
   MINIMUM SPLICE LENGTH FOR ALL #4 BARS IS 16".
   STEEL QUANTITY DOES NOT INCLUDE SPLICE LENGTHS.

- REBAR SHALL BE GRADE 60.

# GENERAL DESIGN NOTES:

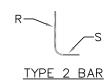
- DRAINAGE SHALL BE AWAY FROM THE WALL.
- THE MINIMUM TOP WIDTH OF THE BACKFILL AGAINST THE WALL SHALL BE EQUAL TO OR GREATER THAN THE BACKFILL HEIGHT.
- •MAXIMUM FOOTING CONTACT PRESSURE IS 900 psf/ft.

<u>DESIGN STRENGTHS:</u> WORKING STRESS DESIGN

CONCRETE  $f_c = 4,000 \text{ psi}$  STEEL  $f_s = 24,000 \text{ psi}$  (GRADE 60)

WALL DESIGN LOADING: 313 STANDARD - LATERAL EARTH PRESSURE VALUES, SEE SECTION IV OF THE FIELD OFFICE TECHNICAL GUIDE.

- \*MANURE LOAD INSIDE = 65 psf/ft.
- •SOIL BACKFILL LOAD OUTSIDE = 60 psf/ft. AND 85 psf/ft.
- NO HORIZONTAL SURCHARGE ADDED.
- \*SOIL BACKFILL DENSITY = 110 pcf.
- \*WATER TABLE MUST BE BELOW THE FOOTING ELEVATION



STEEL SCHEDULE

MARK	SIZE	TYPE	R	S	LENGTH
Α	4	STR			3'-6"
В	4	STR			
* C	4	2	2'-0"	9"	2'-9"
* D	4	STR			3'-9"
L	4	2	2'-0"	9"	2'-9"
L1	4	STR			3'-9"

\* MARK C & D BARS MAY BE COMBINED TO AVOID SPLICE. THEN MARK C BAR IS  $4'-3" \times 9"$ .

## NOTES:

- 1. FOR FROST PROTECTION, A 2-FOOT BACKFILL IS REQUIRED.
- 2. DIMENSIONS ARE TO THE REINFORCING BAR SURFACE.

ADAPTED AND MODIFIED FROM STANDARD DRAWING # PA-020D





(877) 323-6603

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					2024-12-16 REVIEW SET	COMMENTS
					2024-12-16	DATE
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	- 10					

**Eric Stevens** 

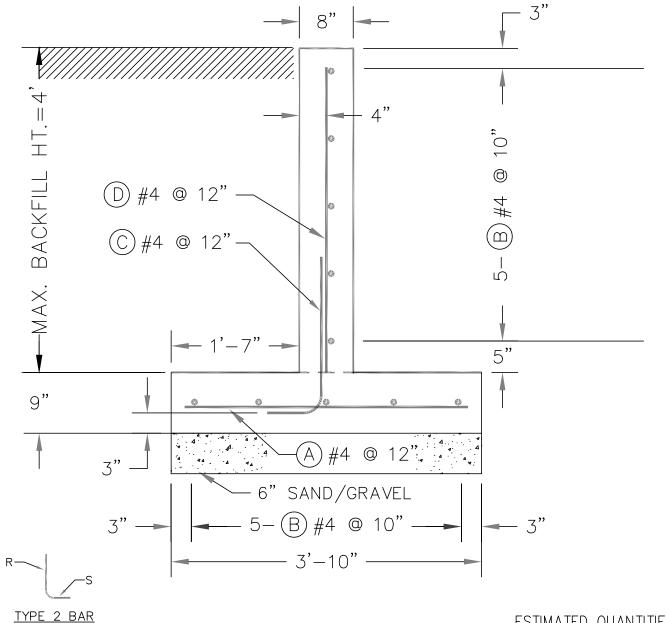
**BUILDING DETAILS** 

STEVENS FARM PROJECT

2025-01-31 Project No.:

Sheet No.:

C-510



TOTAL

LENGTH

LENGTH

3'-6"

2'-9"

3'-9"

2'-9"

3'-9"

# ESTIMATED QUANTITIES

CONCRETE	(0.21 CU.YDS./LIN.FT.)	CU. YDS.
STEEL	(20.67 FT./LIN. FT.)	_ FT.
STEEL	(35.0 FT./CORNER)	_ FT.

· CONCRETE SHALL MEET PA 313 OR 561 SPECIFICATION REQUIREMENTS.

MINIMUM SPLICE LENGTH FOR ALL #4 BARS IS 16". STEEL QUANTITY DOES NOT INCLUDE SPLICE LENGTHS.

REBAR SHALL BE GRADE 60.

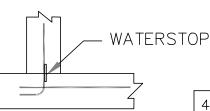
TOTAL LENGTH OF WALL \_ FT.

# C.J=CONSTRUCTION JOINT

LIQUID-TIGHT JOINT \_ YES \_ NO

# LIQUID-TIGHT JOINT OPTIONS

- 1) NON-METALIC WATERSTOP (PVC)
- 2) HYDROPHILIC WATERSTOP



# CONSTRUCTION JOINT OPTIONS

- 1. IF SLAB AND WALL ARE POURED SEPARATELY, THE SLAB SURFACE MUST BE THOROUGHLY CLEANED WITH WATER AND A WIRE BRUSH. THE SURFACE OF THE JOINT SHALL BE KEPT MOIST FOR AT LEAST 1 HOUR PRIOR TO PLACEMENT OF NEW CONCRETE.
- 2. THE SLAB AND WALL MAY BE POURED AT THE SAME TIME ELIMINATING THE NEED FOR A CONSTRUCTION JOINT.

4-FOOT WALL CORNER DETAILS	SFF PA-025
SLAB CORNER DETAILS	SEE PA-023
RESTRAINING SLAB OPTIONS	SEE PA-024

# GENERAL DESIGN NOTES:

- \*DRAINAGE SHALL BE AWAY FROM THE WALL.
- \*THE MINIMUM TOP WIDTH OF THE BACKFILL AGAINST THE WALL SHALL BE EQUAL TO OR GREATER THAN THE BACKFILL HEIGHT.
- \*MAXIMUM FOOTING CONTACT PRESSURE IS 900 psf/ft.

DESIGN STRENGTHS: WORKING STRESS DESIGN

CONCRETE  $f_c = 4,000 \text{ psi}$ STEEL  $f_s = 20,000 \text{ psi}$  (GRADE 40)

WALL DESIGN LOADING: 313 STANDARD - LATERAL EARTH PRESSURE VALUES, SEE SECTION IV OF THE FIELD OFFICE TECHNICAL GUIDE.

- •MANURE LOAD INSIDE = 65 psf/ft.
- \*SOIL BACKFILL LOAD OUTSIDE = 60 psf/ft. AND 85 psf/ft.
- \*NO HORIZONTAL SURCHARGE ADDED.
- •SOIL BACKFILL DENSITY = 110 pcf.
- WATER TABLE MUST BE BELOW THE FOOTING ELEVATION

# WALL RESTRAINT REQUIREMENTS

- 5" THICK SLAB, SAFETY FACTOR AGAINST SLIDING 1.5 MIN

BACKFILL HEIGHT	SLAB LENGTH	SLAB LENGTH
(OUTSIDE LOAD)	NO INSIDE LOAD	FULL INSIDE LOAD
4 FEET	27 FEET	NO SLAB
3 FEET	7 FEET	NO SLAB
2 FEET	NO SLAB	NO SLAB
1 FOOT	NO SLAB	2 FOOT **
0 FOOT	NO SLAB	2 FOOT **

\*\* MINIMUM SLAB LENGTH OF 2 FEET REQUIRED AND MUST BE TIED INTO THE WALL FOOTING. SLAB POURED WITH WALL FOOTING:

#3 BARS @ 18" SPACING (MINIMUM REINFORCEMENT) SLAB NOT POURED WITH WALL FOOTING:

#3 DOWEL BARS - 3'-0" LENGTH @4'-0" SPACING

\* MARK C & D BARS MAY BE COMBINED TO AVOID SPLICE. THEN MARK C BAR IS 4'-3" x 9".

#4 BARS, TOTAL LENGTH

9"

\_\_\_

9"

STEEL SCHEDULE

2'-0"

\_\_\_

2'-0"

# NOTES:

MARK

Α

\* C

\* D

L1

SIZE

4

4

4

4

TYPE

STR

STR

2

STR

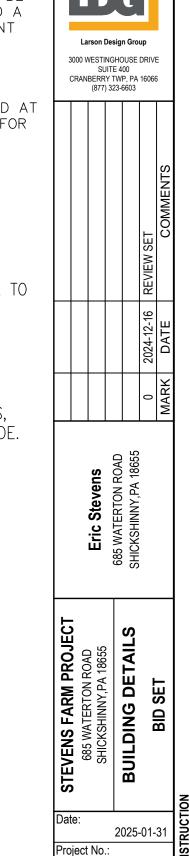
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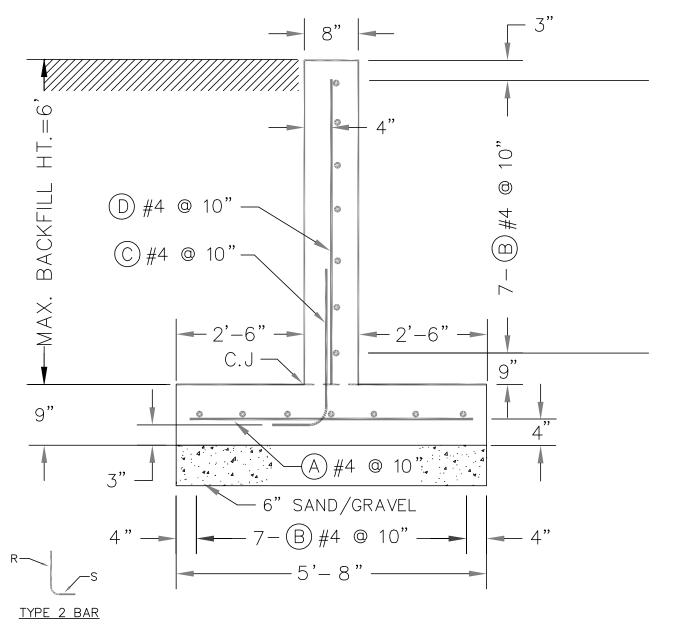
STR

- 1. FOR FROST PROTECTION, A 2-FOOT BACKFILL IS REQUIRED.
- DIMENSIONS ARE TO THE REINFORCING BAR SURFACE.

# 4' T-WALL WITHOUT SURCHARGE SCALE: N.T.S

Sheet No.:





TOTAL

LENGTH

LENGTH

5'-3"

3'-3"

5'-9"

2'-9"

5'-9"

# ESTIMATED QUANTITIES

CONCRETE	(0.21 CU.YDS./LIN.FT.)	CU. YDS.
STEEL	(20.67 FT./LIN. FT.)	_ FT.
STEEL	(35.0 FT./CORNER)	_ FT.

CONCRETE SHALL MEET PA 313 OR 561 SPECIFICATION REQUIREMENTS.

MINIMUM SPLICE LENGTH FOR ALL #4 BARS IS 16".
MINIMUM SPLICE LENGTH FOR ALL #5 BARS IS 17".
STEEL QUANTITY DOES NOT INCLUDE SPLICE LENGTHS. SUBSTITUTION OF GRADE 60 BARS IS PERMITTED.

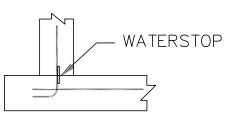
TOTAL LENGTH OF WALL \_ FT.

# C.J=CONSTRUCTION JOINT

LIQUID-TIGHT JOINT \_ YES \_ NO

# LIQUID-TIGHT JOINT OPTIONS

- 1) NON-METALIC WATERSTOP (PVC)
- 2) HYDROPHILIC WATERSTOP



# CONSTRUCTION JOINT OPTIONS

- 1. IF SLAB AND WALL ARE POURED SEPARATELY, THE SLAB SURFACE MUST BE THOROUGHLY CLEANED WITH WATER AND A WIRE BRUSH. THE SURFACE OF THE JOINT SHALL BE KEPT MOIST FOR AT LEAST 1 HOUR PRIOR TO PLACEMENT OF NEW CONCRETE.
- 2. THE SLAB AND WALL MAY BE POURED AT THE SAME TIME ELIMINATING THE NEED FOR A CONSTRUCTION JOINT.

6-FOOT WALL CORNER DETA	ILS SEE PA-027A
SLAB CORNER DETAILS	SEE PA-023
RESTRAINING SLAB OPTIONS	SEE PA-024

# GENERAL DESIGN NOTES:

- DRAINAGE SHALL BE AWAY FROM THE WALL.
- THE MINIMUM TOP WIDTH OF THE BACKFILL AGAINST THE WALL SHALL BE EQUAL TO OR GREATER THAN THE BACKFILL HEIGHT.
- MAXIMUM FOOTING CONTACT PRESSURE IS 1,220 psf/ft.

DESIGN STRENGTHS: WORKING STRESS DESIGN

STEEL  $f_s = 20,000 \text{ psi}$  (GRADE 40) CONCRETE  $f_c = 4,000 \text{ psi}$ 

WALL DESIGN LOADING: 313 STANDARD - LATERAL EARTH PRESSURE VALUES, SEE SECTION IV OF THE FIELD OFFICE TECHNICAL GUIDE.

- •MANURE LOAD INSIDE = 65 psf/ft.
- \*SOIL BACKFILL LOAD OUTSIDE = 60 psf/ft.
- \*NO HORIZONTAL SURCHARGE ADDED.
- \*SOIL BACKFILL DENSITY = 110 pcf.
- \*WATER TABLE MUST BE BELOW THE FOOTING ELEVATION

# WALL RESTRAINT REQUIREMENTS

- 5" THICK SLAB, SAFETY FACTOR AGAINST SLIDING 1.5 MIN

BACKFILL HEIGHT (OUTSIDE LOAD)	SLAB LENGTH NO INSIDE LOAD	SLAB LENGTH FULL INSIDE LOAD
6 FEET 5 FEET 4 FEET 3 FEET 2 FEET 1 FOOT 0 FOOT	20 FEET 6 FEET NO SLAB NO SLAB NO SLAB NO SLAB NO SLAB	NO SLAB NO SLAB 5 FEET ** 5 FEET ** 5 FEET ** 5 FEET **

\*\* MINIMUM SLAB LENGTH OF 5 FEET REQUIRED AND MUST BE TIED INTO THE WALL FOOTING.

SLAB POURED WITH WALL FOOTING:

#3 BARS @ 18" SPACING (MINIMUM REINFORCEMENT) SLAB NOT POURED WITH WALL FOOTING:

#3 DOWEL BARS - 3'-0" LENGTH @ 20" SPACING #4 DOWEL BARS - 3'-0" LENGTH @ 20" SPACING

NOTES:

MARK SIZE

5

4

4

\* C

\* D

L1

1. FOR FROST PROTECTION, A 2-FOOT BACKFILL IS REQUIRED.

STEEL SCHEDULE

2'-3"|1'-0"

2'-0"

S

9"

#4 BARS, TOTAL LENGTH

#5 BARS, TOTAL LENGTH

TYPE

STR STR

STR

STR

DIMENSIONS ARE TO THE REINFORCING BAR SURFACE



685 WATERTON ROAD SHICKSHINNY,PA 18655 **Eric Stevens** STEVENS FARM PROJECT **BUILDING DETAILS** 

3000 WESTINGHOUSE DRIVI

SUITE 400

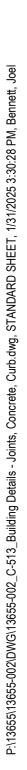
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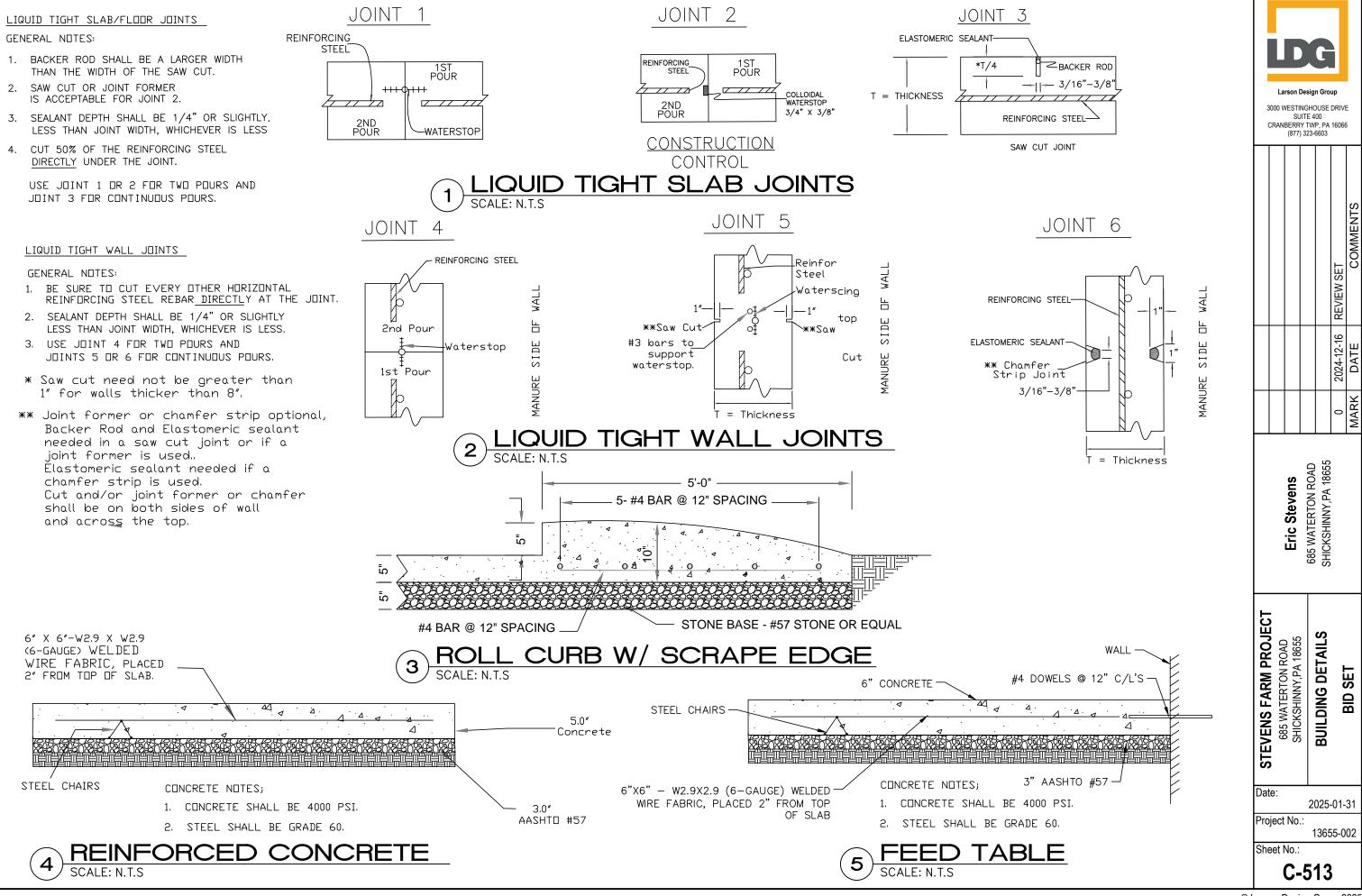
C-512

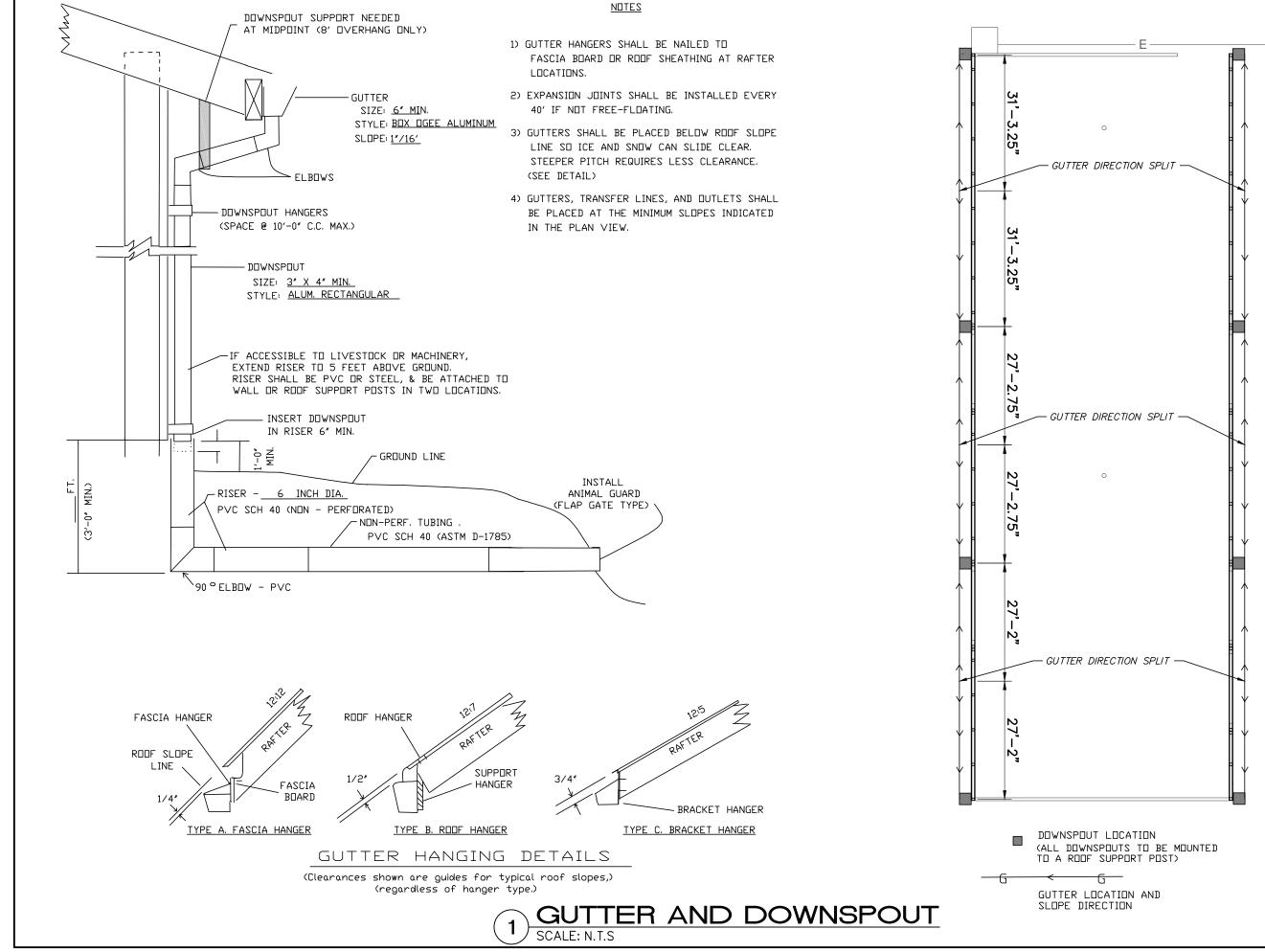
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Larson Design Group

3000 WESTINGHOUSE DRIVE

SUITE 400

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685 WATERTON ROAD SHICKSHINNY,PA 18655

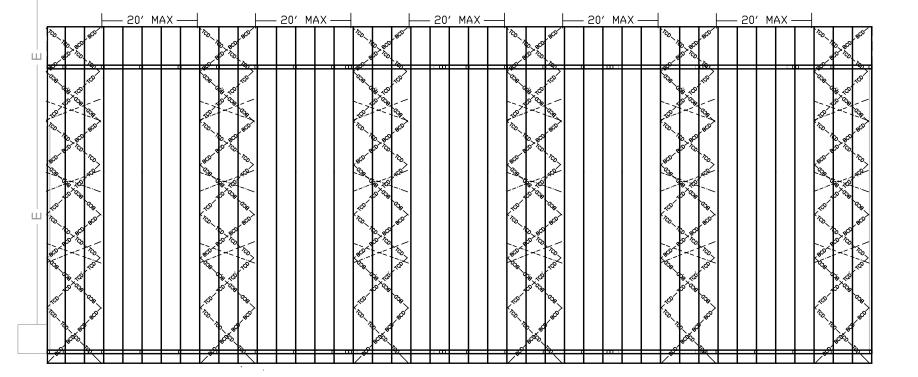
**BUILDING DETAILS** 

2025-01-31

13655-002

**BID SET** 





CONTINUOUS LATERAL BRACING AS PER TRUSS MFG. RECOMMENDATIONS -TCD-TCD-TCD- TOP CHORD DIAGONAL BRACING -BCD-BCD-BCD- BOTTOM CHORD DIAGONAL BRACING

WEB MEMBER CROSS BRACING

P:\13655\13655-002\DWG\13655-002\_C-516\_Building Details - Truss Bracing.dwg, STANDARD SHEET, 1/31/2025 3:31:08 PM, Bennett,

# NOTES:

- 1. CONTINUOUS LATERAL BRACING SHOWN IS FOR A VISUAL REPRESENTATION ONLY; CONTINUOUS LATERAL BRACING LOCATIONS & SPACING ARE REQUIRED BY THE TRUSS MFG & SHOWN ON THE TRUSS DESIGN DRAWING.
- 2. ALL BRACING IS 2" X 4" GRADE MARKED LUMBER.
- 3. ALL CONNECTIONS SHOULD BE MADE WITH 2 16d NAILS. 2-16d NAILS. NO BUTT JOINTS.



685 WATERTON ROAD SHICKSHINNY,PA 18655

**BUILDING DETAILS** 

2025-01-31

13655-002

**BID SET** 

**Eric Stevens** 

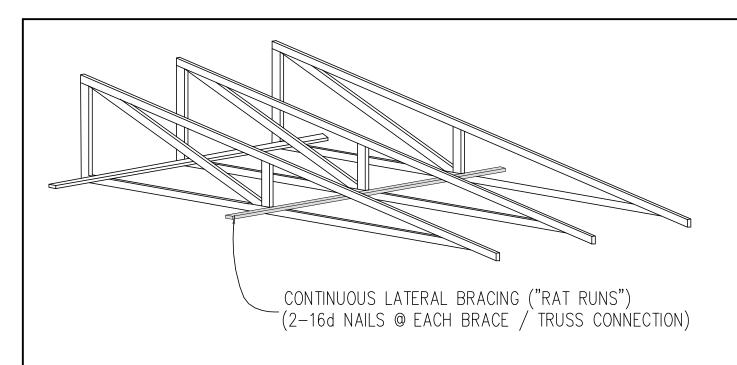
STEVENS FARM PROJECT

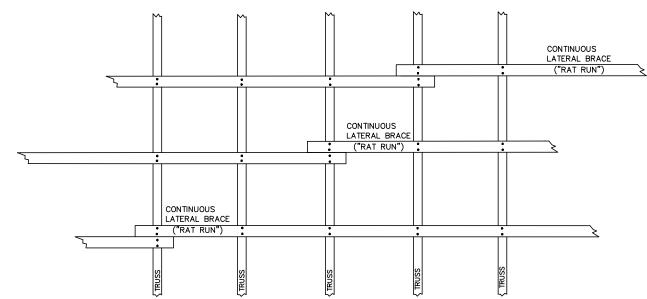
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Project No.:

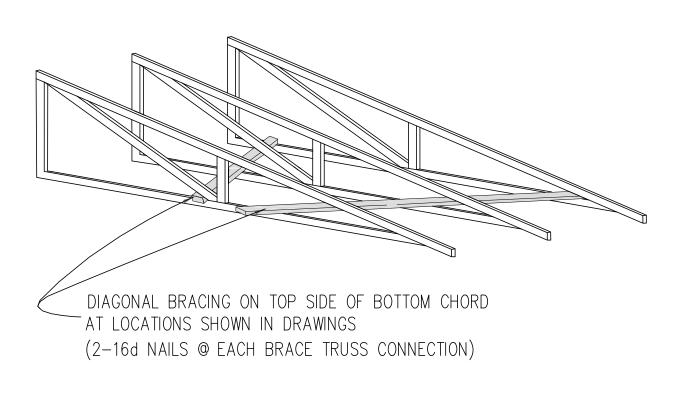
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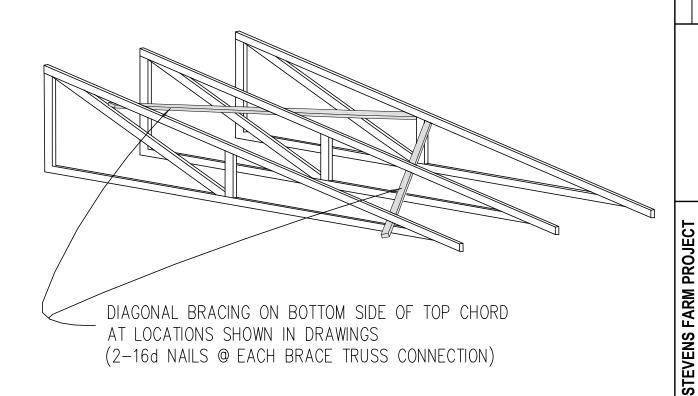
685 WATERTON ROAD SHICKSHINNY,PA 18655





JOINTS IN CONTINUOUS LATERAL BRACES SHALL BE STAGGERED, SO THEY DO NOT LINE UP WITH THE NEXT TRUSS. AT A JOINT, EACH BOARD SHALL EXTEND FULLY PAST THE TRUSS, TO ALLOW FOR A TWO NAIL CONNECTION. THESE BRACES ARE AS PER TRUSS MFG. REQUIREMENTS, SHOWN ON THE TRUSS DESIGN.





CORD AND DIAGONAL BRACING
SCALE: N.T.S

2025-01-31 13655-002

685 WATERTON ROAD SHICKSHINNY,PA 18655

**BUILDING DETAILS** 

**BID SET** 

**Eric Stevens** 

C-517

Project No.:

Sheet No.:

3000 WESTINGHOUSE DRIVE SUITE 400 CRANBERRY TWP, PA 16066 (877) 323-6603

2X4X12 BLOCK ~UNDER BRACE

LATERAL BRACE ("RAT RUN")

THE CROSS BRACE SHALL CONNECT

AT LEAST (3) TRUSSES,

(2-16d NAILS @ EACH MEMBER/BLOCK)

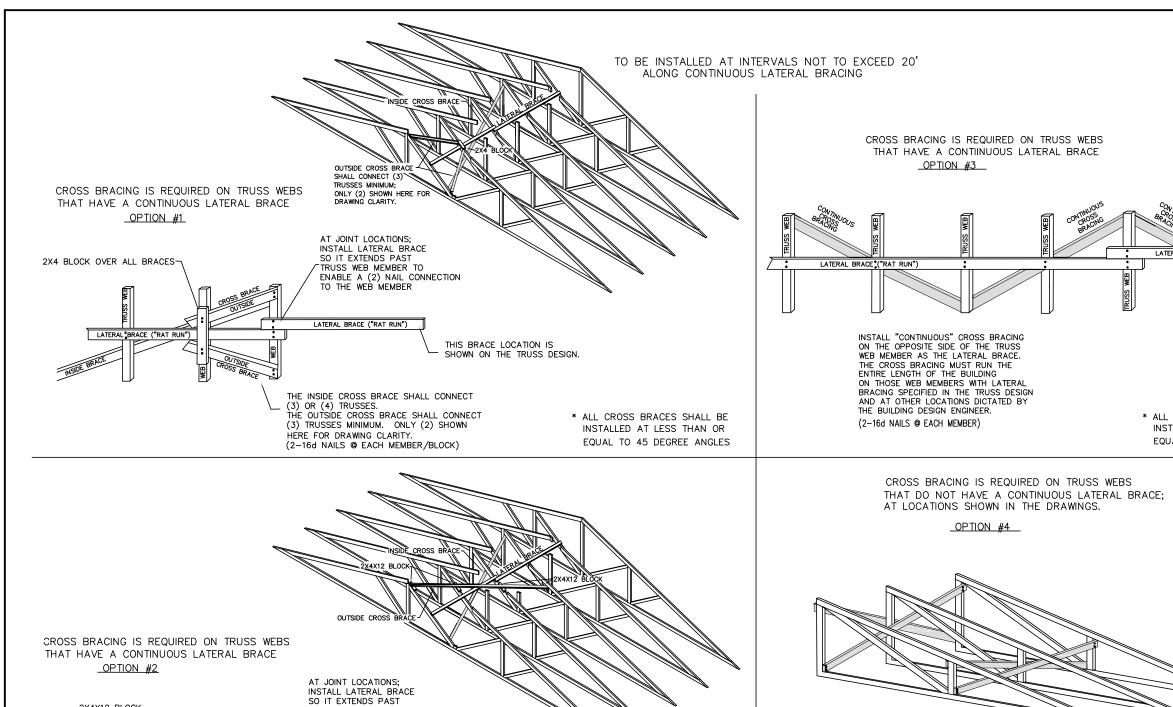
TRUSS WEB MEMBER TO

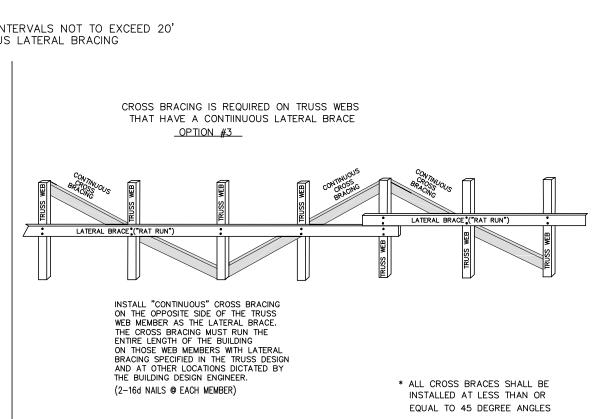
LATERAL BRACE ("RAT RUN")

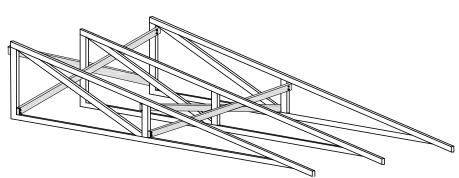
UNDER BRACE

ENABLE A (2) NAIL CONNECTION TO THE WEB MEMBER

THIS BRACE LOCATION IS SHOWN ON THE TRUSS DESIGN.







CROSS BRACING ON BOTH SIDES OF TRUSS WEBS AT LOCATIONS SHOWN WHERE THERE IS NOT A LATERAL BRACE ("RAT RUN") LOCATED ON A TRUSS WEB MEMBER, DICTATED BY THE BUILDING DESIGN ENGINEER. THE CROSS BRACE SHALL CONNECT AT LEAST (3) TRUSSES,

(2-16d NAILS @ EACH MEMBER/BLOCK)

\* ALL CROSS BRACES SHALL BE INSTALLED AT LESS THAN OR EQUAL TO 45 DEGREE ANGLES



\* ALL CROSS BRACES SHALL BE

INSTALLED AT LESS THAN OR

EQUAL TO 45 DEGREE ANGLES

685 WATERTON ROAD SHICKSHINNY,PA 18655 Stevens Eric STEVENS FARM PROJECT **BUILDING DETAILS** 685 WATERTON ROAD SHICKSHINNY,PA 18655 SE 읆 Date: 2025-01-31

Larson Design Group 3000 WESTINGHOUSE DRIVE

SUITE 400

CRANBERRY TWP, PA 16066

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C-518

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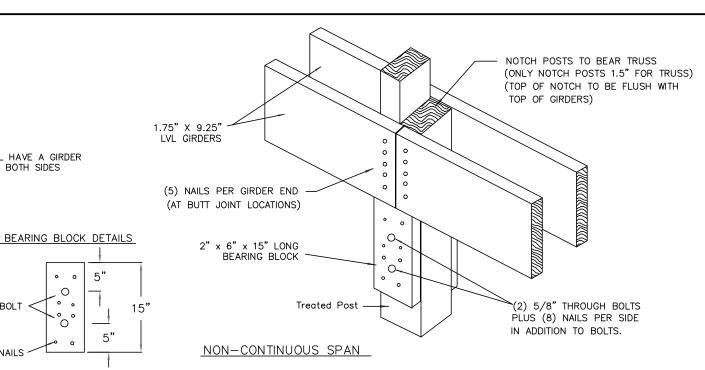
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1.75" X 9.25"

LVL GIRDERS

(10) NAILS PER SIDE

2" x 6" x 15" LONG BEARING BLOCK



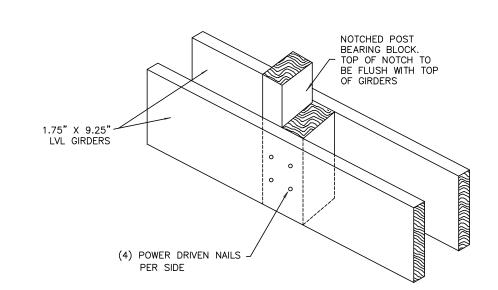
(2) INDIVIDUAL BOARDS

# OPTIONS FOR TRUSS CONNECTION BETWEEN POSTS

\* NO POST SHALL HAVE A GIRDER

BUTT JOINT ON BOTH SIDES

5/8" BOLT

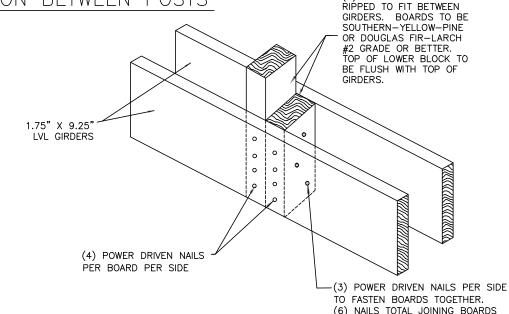


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CONTINUOUS SPAN

Treated Post



# CONSTRUCTION NOTES

NOTCH POSTS TO BEAR TRUSS

(2) 5/8" THROUGH BOLTS

PLUS (8) NAILS PER SIDE

IN ADDITION TO BOLTS.

TOP OF GIRDERS)

(ONLY NOTCH POSTS 1.5" FOR TRUSS)

(TOP OF NOTCH TO BE FLUSH WITH

- 1. Bolts shall be installed in the middle of the girder and support block.
- 2. All nails shall be power driven: .131" Diameter x 3.25" Long (Min.).
- 3. LVL's need to be supported every 2' as per the LVL Manufacturer; A single block, ripped to fit, between the LVL's will suffice. Install (4) power driven nails per side from LVL into the blocking.

# FASTENER REQUIREMENTS AT GIRDER AND POST CONNECTIONS

C-519

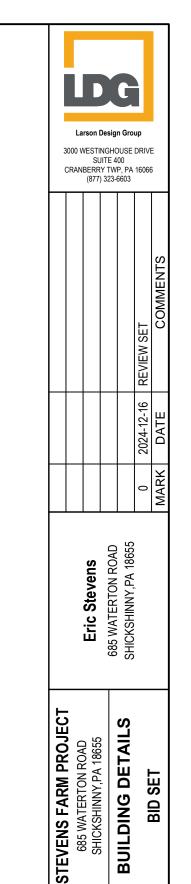
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Date:

Project No.:

Sheet No.:



C-520

2025-01-31

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Date:

Project No.:

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0 MARK

685 WATERTON ROAD SHICKSHINNY,PA 18655

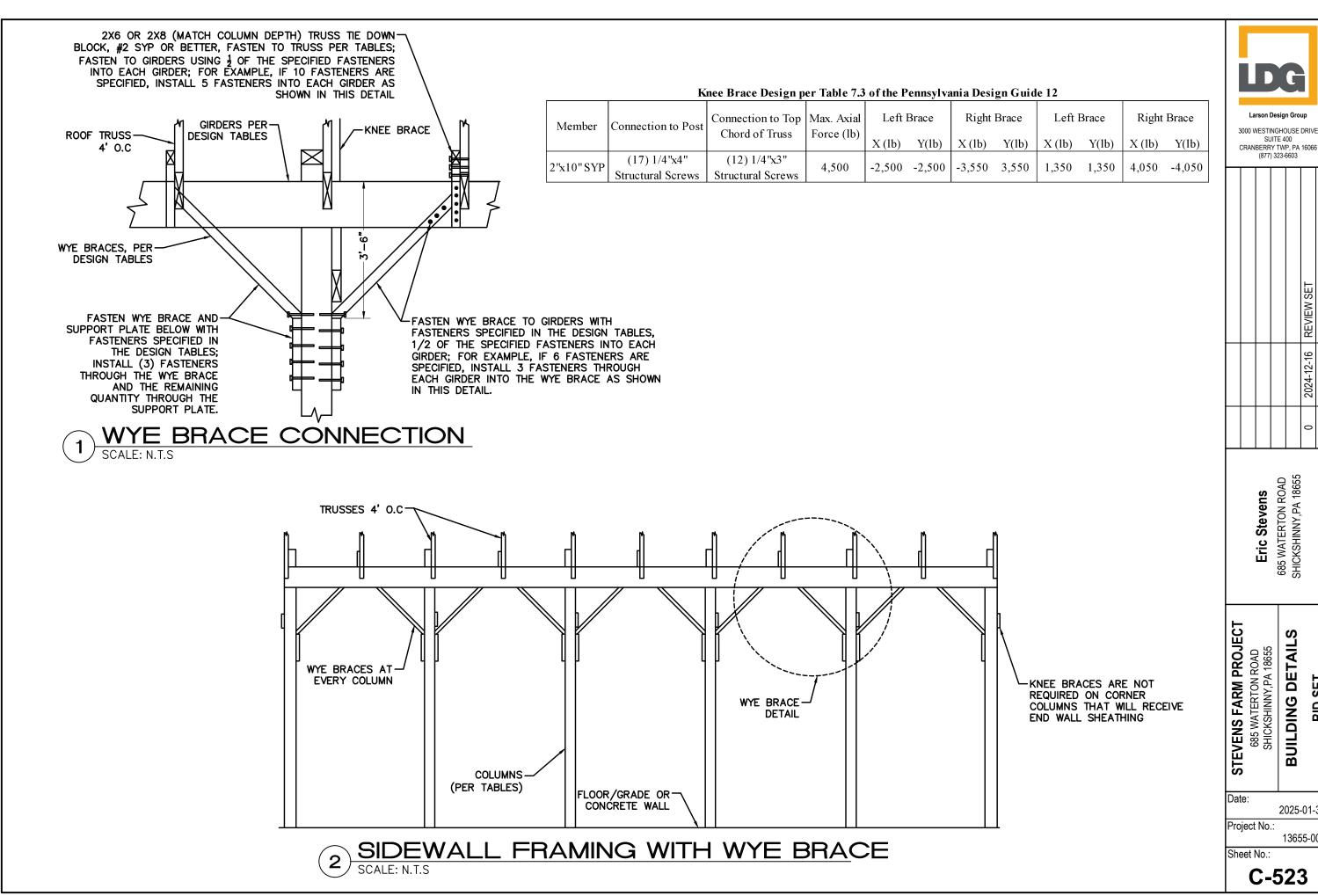
**BUILDING DETAILS** 

2025-01-31

13655-002

**BID SET** 

NOT FOR CONSTRUCTION



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SUITE 400

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**BUILDING DETAILS** 

2025-01-31

13655-002

SET

BD